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DIVISION OF WATER, ENVIRONMENT AND FORESTRY TECHNOLOGY

Report to the

WATER RESEARCH COMMISSION

on

COMPARISON OF PREDICTED SECONDARY DILUTIONS TO MEASURED FIELD DATA AND THE DETERMINATION OF PROTOTYPE DIFFUSION COEFFICIENTS

by

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SCOPE

Between 1991 and 1993 the CSIR conducted full-scale field exercises at three deep sea outfalls along the South African coast to verify the initial dilution prediction methods applied in South Africa. During these exercises the transport of the waste field was also monitored.

These data, together with other related field data, collected over a period of 10 years along the South African coastline, were used to assess the accuracy and applicability of the methods and controlling parameters, e.g. diffusion coefficients, generally used in South African to predict achievable secondary dilutions and subsequent impact at distant locations.

The results of this assessment are presented in this report. The report was compiled by W A M Botes from WAM Technology cc. and Susan Taljaard (CSIR).

A VAN TONDER COASTAL DEVELOPMENT AND MARINE RESOURCES

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EXECUTIVE SUMMARY

The aim of this project is to compare actual secondary dilutions obtained from field data with predicted secondary dilutions, as well as to determine prototype diffusion coefficients at a number of areas along the South African coast to verify the applicability of a standard prediction method used in South Africa for marine outfall design purposes.

The total achievable dilution which can be expected at a distant location is the product of the initial dilution, secondary dilution, dilution due to the die-off of microbiological organisms and the chemical/biological dispersion of non-conservative substances. The initial dilution is the dilution achieved by the entrainment of seawater at the periphery of the plume during the rise of the buoyant effluent from the diffuser to the surface of the sea, thereby reducing pollutant concentrations in the effluent. The subsequent transport of the effluent (waste field) away from the initial surfacing plume, referred to as the 'boil', brings about further reduction of the pollutant concentrations. This process is generally referred to as secondary dilution, which is caused by turbulence, eddies and shears, causing further entrainment/mixing of seawater. Together with chemical and biological dispersion of non-conservative substances and the die-off or decay of certain organisms during the transport of the waste field (dilution due to decay), the initial dilution and secondary dilutions determine the ultimate concentration of pollutants, originating from an effluent, and the subsequent impact on the designated uses at any location away from the discharge location.

From 1991 to 1993 the CSIR conducted full-scale field tests on three outfalls on behalf of the Water Research Commission to verify the methods used for the prediction of initial dilution. These sites were chosen to represent typical coastal conditions and various types of deep sea outfalls in South Africa. A tracer material, Rhodamine-B, was introduced to the outfall system. The concentration of the tracer was then measured in the system and in the initial surface plume (referred to as the 'boil') to determine the achievable initial dilutions. Although not part of those studies, the growth and dispersion of the moving waste field was also monitored by circumnavigation and the sampling of tracer concentrations in the moving waste field.

During 1995 the CSIR was commissioned by the Water Research Commission to assess the applicability of the methods used for prediction of secondary dilutions to South African near-shore conditions, using the available data from these field tests and others. Due to the diversity of near-shore conditions along the South African coastline, it was expected that the dispersion characteristics, i.e. the diffusion coefficients, might be site or region specific.

Because the dispersion of a waste field, after surfacing of the plume, is a spatial and time dependent process, the following data are required to predict behaviour and subsequent impact of the moving waste field:

- ambient current velocities;
- frequency of occurrence of these current speeds and directions;
- frequency of occurrence of wind speeds and directions;
- spatial behaviour of the current, i.e. the near-shore circulation patterns;
- typical diffusion coefficients.

The accuracy of the predicted secondary dilutions depends on the accuracy and completeness of the above-mentioned data sets. For both analytical and numerical methods the diffusion coefficient has to be estimated from experience or relevant data if prototype tests are not conducted at a specific location. The diffusion coefficient cannot be 'modelled' due to the complex physical processes causing the eddy diffusion.

The physical measurements of the dispersion characteristics are a time consuming and costly operation and until present, a 'general' dissipation parameter, referred to as a α -value, of $0.0005~\text{m}^{2/3}~\text{s}^{-1}$ was applied to determine the secondary dilutions for all deep sea outfalls in South Africa, based on the proposals of several researchers and from a once-off field exercise conducted on the Natal coast. The prediction method basically relates to the method of Brooks (1960) applying the so-called '4/3 law', considered to be applicable to 'open sea' conditions.

Various theories can be used to estimate the diffusion coefficients for a specific site, including a procedure whereby 'drogue pair' measurements are used. Between 1981 and 1992, the CSIR undertook numerous studies at a number of coastal development areas to determine the feasibility of ocean outfalls as options for sewage disposal. These studies included the Lagrangian recording of currents, using surface and sub-

surface drogues. The path of the drogues were recorded by accurate position fixing in time and space. Drogue data collected were from Noordwesbaai (on the west coast), Hout Bay, False Bay, Vlees Bay and East London.

This valuable source of data is also assessed in this report to estimate the magnitude of the diffusion coefficient at various coastal regions. Tracer measurements, collected at Richards Bay, Viees Bay and Hout Bay, were also assessed to estimate diffusion coefficients on the days of the exercises.

The comparison between predicted and measured secondary dilutions yielded the following:

- Richards Bay. Using a α-value of 0,0005 m^{2/3} s¹, the predicted secondary dilutions at Richards Bay were about 2,5 times less than the measured dilutions. Previous studies in the area (Pearce, 1969), had also indicated that a α-value of 0,0005 m^{2/3} s⁻¹ can be considered as conservative. The actual α-value on the day of the exercise was 0,0017 m^{2/3} s⁻¹, applying the '4/3 law'. The predicted waste field width was also about two times less than the measured width.
- Viees Bay. The predicted secondary dilutions, using a α -value of 0,0005 m^{2/3} s⁻¹, were slightly less than the measured values. The predicted waste field width was about two times less than the measured width, possibly associated with the estimation of the angle between the current direction and the diffuser orientation (i.e. θ). This angle is used to calculate the effective diffuser width (i.e. the initial waste field width, b, where b = w sin θ). The estimation of the effective diffuser width also affects the prediction of secondary dilutions, but not as drastically as for the prediction of waste field width. For a relatively short diffuser, the effective diffuser width would be less affected by an inclined current considering the width of the initial waste field ('boil'), relative to the length of the 'boil'.
- Hout Bay. Changing current conditions, which caused reversal of the moving waste field, limited the time period during which concentration measurements after the initial dilution process (i.e. after surfacing of the plume) could be conducted at Hout Bay. Due to the 'near stagnant' conditions, measured secondary dilutions were only limited to about 1,5 after 36 minutes, which were slightly less than the predicted secondary dilutions.

The estimation of the α -values from the selected drogue data yielded the following:

Although the median α -value of 0,00052 m^{2/3} s⁻¹ for all sites compare well with the generally suggested value of 0,0005 m^{2/3} s⁻¹ for open sea conditions, the individual values for certain areas, for example Noordwesbaai differ substantially from the general α -value of 0,0005 m^{2/3} s⁻¹.

The effect of current velocity on the turbulent behaviour of the receiving water and subsequent α -value is shown, where the α -value for currents less than 0,1 m s⁻¹ is 0,00046 m^{2/3} s⁻¹ compared to 0,00083 m^{-2/3} s⁻¹ for currents greater than 0,2 m s⁻¹. In sheltered areas, where currents are weaker, a value of less than 0,0005 m^{2/3} s⁻¹ should be used for a conservative approach, whereas in a more dynamic environment, for example east coast conditions (Richards Bay), values greater than 0,0005 m^{2/3} s⁻¹ can be applied.

The estimated median α -value for sub-surface currents (-5 m water depth) is approximately 20 percent lower than the estimated value for surface currents. This is important especially when conducting performance evaluations of deep sea outfalls by measuring of actual dilutions using tracer material in an effluent, as most of the samples are taken close to the sea surface.

The influence of the length scale (L_x) obtained from the drogue data illustrated that for a length scale of between 80 and 120 m the α -value is less than two times the α -value for a length scale of less than 40 m.

The following *conclusions* and *recommendations* can be drawn from the findings of this project:

- Results from this project indicate that the 'general' procedure applied in the prediction of achievable secondary dilutions for deep sea outfall along the South African coast, i.e. the method of Brooks (1960), assuming the '4/3 law' and a α-value of 0,0005 m^{2/3} s⁻¹ does not always apply.
- Although the median α -value of 0,0005 m^{2/3} s¹, obtained for grouping all available drogue measurements, confirmed the 'general' α -value suggested by

numerous authors, the α -values estimated for the individual sites, clearly showed the diversity of conditions along the 3 000 km South African coastline.

- Based on the findings of this report the use of a 'general' α-value is not recommended in determining the secondary dilutions and subsequent impact of an effluent waste field at distant locations in South Africa. The most typical values for certain areas should be considered and if detailed field measurements cannot be obtained to establish the actual diffusion coefficients, at least a range of values should be considered. The diffusion coefficients and α-values determined in this report could be used as a rough guideline.
- Although the prediction of achievable secondary dilutions and subsequent impact at distant locations is considered appropriate for planning and feasibility phases of an outfall project, taking into account the near shore processes and the coastline configuration, it is strongly recommended that field measurements be conducted to obtain more accurate site specific diffusion characteristics of the area for the optimisation of the outfall during the detailed design phase.
- Irrespective of the procedure or method used to predict the behaviour of a moving waste field, i.e. analytical methods or numerical modelling, a thorough understanding of the physical processes, the sensitivity and limitations of the prediction method or procedure to all variables, as well as the applicability of a certain methods to a specific area or near shore conditions is essential before any quantitative conclusions can be made with regard to the detailed outfall design. A simple equation or a 'black box' approach is therefore not always considered to be sufficient in providing reliable quantitative results on achievable secondary dilutions, especially taking into account the consequences of detrimental impact in terms of ecological, health and economical aspects, both in the short and long term.

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Comparison of predicted secondary dilutions to measured field data and the determination of prototype diffusion coefficients.

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viii

TABLE OF CONTENTS

Ackı Tabi List	cutive nowled		nts	i ii vii xiii ix xi
1	INT	RODU	CTION	1
2.	LITE	ERATU	RE REVIEW	4
3.	MAT	ΓERIAL	S AND METHODS	14
	3.1	Data	3	14
	3.2	Met	nods	22
4.	RES	ULTS	AND DISCUSSION	24
	4.1	Com Mea	parison of Predicted Secondary Dilutions to sured Field Data	24
		4.1.1	Richards Bay	24
		4.1.2	Vlees Bay	29
		4.1.3	Hout Bay	33
	4.2	Dete (and	rmination of Prototype Diffusion Coefficients α-values)	38
		4.2.1	Determination of diffusion coefficients (and α-values) based on drogue measurements	38
		4.2.2	Determination of diffusion coefficients (and α-values) based on tracer measurements	47
j	CON	CLUSIC	ON AND RECOMMENDATIONS	51
REFE	RENC	ES		53
	NDIX / NDIX I		Selected Drogue Data Detailed Statistical Analyses and Plots of Data	

LIST OF TABLES

TABLE 1a.	Measured secondary dilutions versus distance measured for Richards Bay	25
TABLE 1b.	Measured waste field width versus distance for Richards Bay	26
TABLE 1c.	Predicted secondary dilutions versus distance for Richards Bay	27
TABLE 1d.	Predicted waste field width (L_x) versus distance for Richards Bay	27
TABLE 2a.	Measured secondary dilutions versus distance measured for Vlees Bay	30
TABLE 2b.	Waste field width versus distance measured for Viees Bay	30
TABLE 2c.	Predicted secondary dilutions versus distance for Vlees Bay	31
TABLE 2d.	Predicted waste field width (L _x) versus distance for Vlees Bay	32
TABLE 3a.	Measured secondary dilutions versus time measured for Hout Bay	34
TABLE 3b.	Waste field width versus time measured for Hout Bay	35
TABLE 3c.	Predicted secondary dilutions ($S_{\rm e}$) and waste field width ($L_{\rm x}$) versus time for Hout Bay	36
TABLE 4a.	Median K- and α-values for Noordwesbaai determined for various combinations of conditions	40
TABLE 4b.	Median K- and α-values for False Bay determined for various combinations of conditions	41
TABLE 4c.	Median K- and α-values for Hout Bay determined for various combinations of conditions	41
TABLE 4d.	Median K- and α-values for Vlees Bay determined for various combinations of conditions	41

TABLE 4e.	Median K- and α-values for East London determined for various combinations of conditions	42
TABLE 4f.	Median K- and α -values for all sites (grouped) determined for various combinations of conditions	42
TABLE 5.	The median and exponential fit versus distance of all the K- and α -values calculated from the selected drogue data sets	46
TABLE 6a.	Average and median surface concentrations in the 'boil' (initial) and at a distance for Richards Bay	48
TABLE 6b.	Average and median surface concentrations in the 'boil' (initial) and at a distance for Vlees Bay	48
TABLE 6c.	Average and median surface concentrations in the 'boil' (initial) and at specific times after release for Hout Bay	48
TABLE 7.	Current velocities on the days of the field measurements at the three sites	48
TABLE 8.	Diffusion coefficients and α-values calculated for the three sites at different distances from the discharge location	50

LIST OF FIGURES

Figure 1a.	Richards Bay: Locality map of sea outfall (A-line)	16
Figure 1b.	Richards Bay: Currents measured on 11 September 1991	17
Figure 2a.	Vlees Bay: Locality map of the sea outfall	18
Figure 2b.	Vlees Bay: Currents measured on 10 November 1992	19
Figure 3a.	Hout Bay: Locality map of the sea outfall	20
Figure 3b.	Hout Bay: Currents measured on 12 October 1993	21
Figure 4a.	Surface tracer concentrations (log scale) versus distance from discharge point for Richards Bay on 11 September 1991	24
Figure 4b.	Secondary dilutions (log scale) versus distance from discharge location for Richards Bay on 11 September 1991	25
Figure 4c.	Richards Bay: Transport of the waste field on 11 September 1991	26
Figure 4d.	Predicted and measured secondary dilutions for Richards Bay	28
−igure 4e.	Predicted and measured waste field widths for Richards Bay	28
Figure 5a.	Surface tracer concentrations (log scale) versus distance from discharge location for Vlees Bay On 10 November 1992	29
Figure 5b.	Secondary dilutions (log scale) versus distance from discharge location for Viees Bay on 10 November 1992	29
igure 5c.	Vlees Bay: Transport of the waste field on 10 November 1992	30
igure 5d.	Predicted and measured secondary dilutions for Vlees Bay	32

Figure 5e.	Predicted and measured plume widths for Vlees Bay	33
Figure 6a.	Surface tracer concentrations (log scale) versus time for Hout Bay on 12 October 1993	34
Figure 6b.	Secondary dilutions (log scale) versus time for Hout Bay on 12 October 1993	34
Figure 6c.	Hout Bay: Transport of the waste field on 12 October 1993	35
Figure 6d.	Predicted and measured secondary dilutions for Hout Bay	36
Figure 6e.	Predicted and measured waste field widths for Hout Bay	37
Figure 7.	Relationship between secondary dilution and the dimensionless parameter (p), as well as measured secondary dilutions for Richards Bay, Viees Bay and Hout Bay	37
Figure 8a.	Median of α -values calculated for all sites, in both surface and sub-surface (-5 m) waters at different current velocities	43
Figure 8b.	Median of α -values calculated for all sites, in both surface and sub-surface (-5 m) waters at different water depths	43
Figure 8c.	Median of K-values calculated for all sites, in both surface and sub-surface (-5 m) waters at different current velocities	44
Figure 8d.	Median of α -values calculated for all sites, in both surface and sub-surface (-5 m) waters at different current velocities	44
Figure 8e.	Median of α -values calculated for all sites, in both surface and sub-surface (-5 m) waters at different length scales	45
Figure 8f.	Median of α-values calculated for all sites, in both surface and sub-surface (-5 m) waters at different distances	45

1. INTRODUCTION

The aim of this project is to compare actual secondary dilutions obtained from field data with predicted secondary dilutions, as well as to determine prototype diffusion coefficients at a number of areas along the South African coast to verify the applicability of a standard prediction method used in South Africa for marine outfall design purposes.

When a buoyant effluent is discharged into the sea, the reduction of concentrations of constituents is brought about by various physical, chemical and biological processes. The physical dilution of an effluent can be considered as two distinct processes, i.e.

- i. The initial dilution process where a buoyant plume rise from the diffuser or an open end pipeline to the surface of the sea. The dilution is brought about by the entrainment of seawater during the rise of the plume. The influencing parameters are the buoyant and momentum flux of the jet, the ambient currents and the density structure of the receiving water column. The dilution obtained from this process can be optimised by adapting the diffuser design for a certain ambient environment.
- ii. Secondary dilution or subsequent dilution (after dissipation of the energy during the initial dilution phase) where the plume (waste field) is transported by ocean currents to distant locations. During the transport of the waste field, mixing occurs due to eddies which arise from various physical processes, also referred to as eddy diffusion. Unlike during the initial dilution process, this cannot be influenced by the design of the outfall and is primarily dependent on the near-shore oceanographic conditions.

In a previous research project financed by the Water Research Commission the applicability of different methods for the prediction of initial dilution was investigated at three outfall sites, i.e. Richards Bay, Vlees Bay and Hout Bay (WRC, 1994). This project will focus on the methods and controlling parameters used in the prediction of secondary dilution.

From the planning to the final design phase of a deep sea outfall, the estimation of the secondary dilutions, and subsequent impact at distant locations, are essential,

irrespective of whether analytical or numerical models are used for the assessment. Detailed field measurement, for this purpose, are time consuming and costly, especially when considering the seasonal variations of physical conditions. Detailed measurements are, therefore, only conducted during the final design phase. For the planning and feasibility phases, secondary dilutions need to be predicted as realistically as possible.

At present a standard prediction method for secondary dilution has been applied for outfall design purposes along the South African coast. It is primarily based on the method developed by Brooks (1960) which defines the diffusion coefficient, a controlling parameter in the determination of secondary dilutions as:

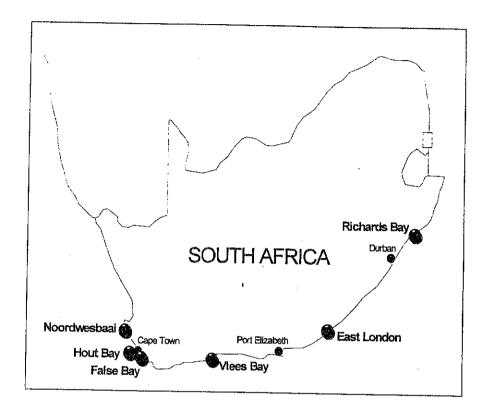
 $K = \alpha L^n$

For open ocean conditions, assumed to be representative of deep sea outfalls, a value of 4/3 is recommended for n, referred to as the '4/3 law'. The α -value assumed for the South African situation is 0.0005 m^{3/2} s⁻¹ (Brooks, 1960; Pearce, 1969).

However, due to the diversity of the coastal processes along the 3 000 km South African coastline, the application of a 'general diffusion' coefficient has caused some concern over the last few years and, therefore, warranted investigation.

Between 1981 and 1994, the CSIR undertook numerous studies at certain coastal development areas to determine the feasibility of ocean outfalls as options for sewage treatment and to investigate initial dilution methods commonly applied in South Africa (i.e. Noordwesbaai, Hout Bay, False Bay, Vlees Bay, East London and Richards Bay). Measurements included the Lagrangian recording of currents, using pairs of surface and sub-surface drogues and tracer (i.e. Rhodamine B) measurements. The path of the drogues were recorded by accurate position fixing in time and space. These data provide a valuable basis for determining the magnitude of eddy diffusivity at various locations along the South African coastline (i.e. diffusion coefficients). The analysis of this data not only contributes to a better understanding of South African conditions, but will also supplement any future studies on diffusity and the refinement diffusion coefficients elsewhere.

The location of the different sampling sites mentioned throughout this report are indicated below:



This report presents the results from this investigation. The report is structured as follows:

-

- Literature review, where theories and methods, specifically related to deep sea outfall studies are assessed.
- Materials and Methods, where the data used for this project are defined and the methods used in the assessment are outlined.
- Results and Discussion, where measured secondary dilutions are compared with predicted values and where prototype controlling parameters (i.e. diffusion coefficients) in the determination of secondary dilutions, are investigated for a number of areas along the South African coast.
- Conclusion and Recommendations, where the important findings of the study are summarised and recommendations, related to these findings, are outlined.

2. LITERATURE REVIEW

Since the scope of this study was not to evaluate the numerous theories and methods used to determine secondary dilution, but rather to compare field measurements to prediction methods generally applied in South Africa, the literature review will focus on the practical application of diffusion coefficients and secondary dilutions in the near shore environment related to the discharge of long sea outfalls. Theories related specifically to diffusion in the ocean were developed by Okubo (1962), Joseph and Sendner (1958) and Brooks (1960), and were summarised by Yundelson (1967).

Because the dispersion of a waste field after surfacing of the plume is a spatial and time dependent process, the following data are required to predict behaviour and subsequent impact of the moving waste field:

- ambient current velocities;
- ii. frequency of occurrence of these current speeds and directions;
- iii. frequency of occurrence of wind speeds and directions,
- iv. spatial behaviour of the current, i.e. the near shore circulation patterns;
- v. typical diffusion coefficients.

The accuracy of the predicted secondary dilutions depends on the accuracy and completeness of the above-mentioned data sets. For both analytical and numerical methods the diffusion coefficient has to be assumed if prototype tests are not conducted at a specific location. The diffusion coefficient cannot be 'modelled' due to the complex physical processes causing the eddy diffusion.

Numerous studies have been conducted to develop mechanisms and theories to predict the turbulent diffusion and behaviour of a surface waste field when transported away from a discharge location. Researchers who made valuable contributions include Brooks (1960 and 1972), Yudelson (1967) and Pearce (1969). These and many others facilitated in formulating and quantifying complex oceanographic processes in order to provide 'tools' to assist with the design of marine outfalls with regard to the impact which a discharged effluent may have at distant locations.

The theory of turbulent diffusion has been studied since the beginning of the century and numerous field experiments contributed to the development of rational methods which can be applied. Various theories and concepts to define this process were developed:

With reference to Yudelson (1967):

Einstein defined the diffusion coefficient as:

Where σ_x^2 = mean square dispersion (or variance of distribution) of the diffusing particles in one direction after time t.

 Fick's law relates the variation of the concentration C of a substance to eddy diffusivity and mathematically described diffusion as:

$$\frac{\delta C}{\delta t} = K_x \frac{\delta^2 C}{\delta x^2}$$

Where K_x is a function of the distance x.

Brooks (1960) described the diffusion process with regard to the scale of the waste field as:

When the waste field is transported away from a discharge location, it is mixed with the adjacent seawater due to irregular turbulent circulation, super-imposed on the main flow field. This mixing will only occur if the turbulent fluctuations or eddies are smaller than the main flow of the waste field. If the main flow or waste field is smaller than the ocean eddies mixing will not be promoted. Thus in the ocean, the larger the scale of the waste field becomes, the larger the effect of eddies will be on the mixing process. However, this does not apply for molecular diffusion when the mixing of one substance with another relates to the random molecular motion.

For this study the works of Brooks (1960) and Yundelson (1967), reviewed by many other authors, is considered to be the most applicable and relevant when considering a rational method to predict secondary dilutions.

According to Brooks (1960) the linear diffusion law is the simplest for engineering applications and can be expected to be reasonably reliable if the diffusion coefficient (K_x) is considered a variable. He used the basic Fickian equation to determine the dispersion of a waste field, i.e.:

Where C =concentration of the substance being diffused x =a coordinate in the direction of the flow path.

He also assumed that K_x is a function of the length scale (L_x), i.e. the width of the waste field and that vertical mixing as well as longitudinal mixing are negligible and that the main flow (transport) is steady. The die-off of microbiological organisms was not considered as part of the physical mixing during transport, but is handled separately as a 'dilution of decay' which is defined as e^{-kt} , where t can be expressed in terms of the distance x and the current velocity U. Because the nature of the field data used in this project, i.e. physical drogue measurements and tracer (Rhodamine-B) measurements, the dilution due to decay will not be discussed.

Neglecting vertical mixing, yields a more conservative approach although the magnitude of the vertical dilution is several magnitudes lower compared to horizontal diffusion as reported by Koh and Brooks (1975). Typical diffusion coefficients for vertical mixing range from 0,0001 to 0,004 m² s⁻¹. Generally, for a fresh water effluent the relative buoyancy (compared to the ambient water) will curtail the vertical mixing, while vigorous wave action will enhance the vertical mixing.

Considering the assumption that the diffusion coefficient is a function of the size and width of a waste field, the initial horizontal diffusion coefficient (K_o) for a spreading effluent plume which varies with the length scale of the waste field can be expressed as follows:

$$K_0 = \alpha L^n$$

Where L = length scale
α = a dissipation parameter ('variable constant')
n = a power exponent

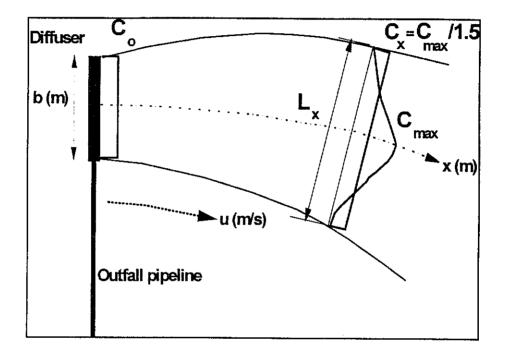
Field measurements by Okubo (1974) and Pearce (1969) concluded that in the near shore environment where the spreading of the waste field is unobstructed n values range from 1.0 < n < 4/3.

The best agreement with open coastal waters according to Okubo (1974) and Pearce (1956) corresponds with a n-value of 4/3 (generally referred to as the '4/3 law'). This value of n applies to waste field widths (L_x) ranging from 10 m to 10 000 m and is generally applicable to deep sea outfalls. In calmer coastal waters the spreading of the waste field becomes curtailed due to limited eddies and confined areas which limit mixing (proximity of the coastline). In these instances the values for n are more likely to be in the range 0,5 to 1,0. In restricted areas such as estuaries and small embayments where flow is homogenous and the lateral mixing is limited by adjacent estuary banks a value of zero can be used for n.

The estimated dilution factor relates to the average concentration at the discharge location (C_0) and the centre line concentration (C_{max}) at a distant location (x).

Assuming a transverse Gaussian distribution (situation is illustrated schematically in the sketch below), the average concentration at a distance (x) from the discharge location is:

$$C_{x} = \frac{C_{\text{max}}}{1.5}$$



The diffuser of the outfall is taken as a steady line source of a width b and the plume is transported by a uniform steady flow u in m s⁻¹. The concentration, C_o, is the concentration at the surface after the initial dilution process had taken place:

$$C_0 = \frac{C_e}{S_t}$$

Where

C_e = concentration in the effluent in the pipe S_i = initial dilution which occurred when the e

initial dilution which occurred when the effluent rises to the surface due to jet momentum and the relative buoyancy of the effluent

The minimum secondary dilution is:

$$S_{\text{emin}} = \frac{C_0}{C_{\text{max}}}$$

For the prediction of the secondary and eddy dilution (S_e) when concentrations are not measured, the initial horizontal diffusion coefficient (K_o) is the key parameter.

For a constant value of K_o , K_o increasing linearly with the length scale and for K_o proportional to $L^{4/3}$, the maximum concentrations along the centre line of a moving waste field, according to Brooks (1960), are:

Description of K _o	$C_{\text{max}}/C_{\text{o}} = 1/S_{\text{e}}$
K₀= constant	$erf\sqrt{\frac{3}{4\beta x/b}}$
$K_o = \alpha L$	$erf\sqrt{\frac{3/2}{(1+\beta x/b)^2-1}}$
$K_o = \alpha L^{4/3}$	erf $\sqrt{\frac{3/2}{(1+2/3\beta x/b)^3-1}}$

Where

 $\beta = 12K_o/ub$

K_o = initial horizontal diffusion coefficient

b = initial plume width (m)

U = uniform current speed (m s⁻¹)

x = distance downstream (m)

 C_{max} = concentration at distance x.

Substituting a dimensionless distance $p = \beta x/b$, these equations to describe the concentrations along the centre line become:

Description of K _o	$C_{\text{max}}/C_{\text{o}} = 1/S_{\text{e}}$
K₀= constant	$erf\sqrt{\frac{3}{4p}}$
K _o = αL	$erf\sqrt{\frac{3/2}{(1+p)^2-1}}$
$K_0 = \alpha L^{4/3}$	erf $\sqrt{\frac{3/2}{(1+2/3p)^3-1}}$

The travel time t can be expressed as u/x which will result in:

$$\beta = 12K_o t/xb$$

$$P = 12K_o t/b^2$$

The error function which is defined as:

erf (X) =
$$\frac{2}{\sqrt{\Pi}} \int_0^x e^{-p^2} dp$$

relates to the solving of the partial differential equations that involve convection and diffusion.

Similarly the plume width (L_x) at distance x along the centre line of the path can be expressed, according to Brooks (1960), as:

Description of K _o	L _x (plume width)
K _o = constant	$b[1 + 2p]^{1/2}$
$K_o = \alpha L$	b[1 + p]
$K_0 = \alpha L^{4/3}$	b[1 + 2/3p] ^{3/2}

For direct onshore conditions this method is conservative, since onshore currents are complex physical processes which are normally deflected, resulting in horizontal and vertical counter currents. Together with the more complex behaviour of onshore ambient currents, other oceanic processes such as wind shear and wave action are not easy to describe mathematically in shallower water. This results in more complex and vigorous mixing processes and, subsequently, larger dilutions. This phenomenon, especially in instances where ambient currents are dominated by the tide, can be simulated better by numerical models during the final design stage of a project.

The values of α depend on the natural turbulence (related to wave, wind and current conditions) and the initial plume width. According to Yudelson (1967) the most common range is between 0,02 and 0,005 cm^{2/3} s⁻¹. Okubo (1974) compared data from numerous experiments and found a good agreement between the diffusion coefficients and length scales for an α -value of 0,0005 m^{2/3} s⁻¹ applying the 4/3 law for open coastal waters. Pearce (1969) also recommended a 'conservative' value of 0,0005 m^{2/3} s⁻¹, which according to him, can be considerably higher in the presence of shearing currents and a severe wave climate. Nine experiments on the Natal coast yielded values ranging from 0,0005 m^{2/3} s⁻¹ to 0,014 m^{2/3} s⁻¹ using data from drift cards, tracers and transport of a waste field. The recommended lower value was preferred. Although it was shown to be conservative, confirmation of this became necessary considering the diversity of the coastal conditions along the South African. The value of α may vary from 0,00001 to 0,03 m^{2/3} s⁻¹ and for more detailed assessments of impacts from sea outfalls, site specific investigations may be a requirement in the future. This would not only be for the determination of the achievable secondary

dilutions, but also for the prediction of the geometry of the plume (i.e. plume width). This is not only a requirement for analytical assessments, but also for the calibration and verification of numerical models.

Prototype diffusion coefficients can be estimated by:

- i. measuring the change in concentration of a tracer material (e.g. Basazol Red or Rhodamine B) over distance and time, measuring the actual change in size of the spreading waste field and through accurate photographic records;
- ii. the recording and statistical analysis of spreading of particles (drift cards);
- iii. studying the separation in time and distance of drogue pairs (floats).

The mean eddy diffusion coefficient from Einstein's definition for a distribution of particles can be estimated as follows:

$$K = \frac{{\sigma_2}^2 - {\sigma_1}^2}{2(T_2 - T_1)}$$

Where σ_1 and σ_2 are the variance of the distributions at times T_1 and T_2 (rate of change of σ^2)

The assumption is that at any time t, the concentration $C_{\rm x}$ at a distance x can be expressed as a normal curve:

$$C_{x} = \frac{C_{0}}{\sigma \sqrt{2\pi} e^{\frac{(x/\sigma)^{2}}{2}}}$$
 Where $\sigma = \sqrt{2Kt}$

Richardson and Stommel's approach (1948) proposed a theory to define neighbour diffusivity of which the concept is based on the separation of the particles in the waste field instead of a unit concentration C_x at a distance x.

Richardson's Law (Yudelson, 1967) expressed the neighbour diffusivity $F(\ell)$ by the following equation:

Where l_0 = the initial separation of the particle at t = 0 l_1 = the separation after time t = T

The length scale should be defined as l_0 if $(l_1 - l_0)$ is relatively small and as $(l_1 - l_0/2)$ if $(l_1 - l_0)$ is large.

Referring to equation (8), the length scale (L) of the waste field can be expressed as $L = 4\sigma$ since 95 % of the lateral concentrations of a substance in the plume will be within the width $(-2\sigma + 2\sigma)$.

Thus, the diffusion coefficient can be expressed in terms of the width of the waste field as follows:

Where w_1 and w_2 are the width at times t_1 and t_2

For drogue pairs it is suggested by Pearce (1967), according to Richardson and Stommel (1948), that the diffusion coefficient can be determined as follows:

$$K = \frac{L_2^2 - L_1^2}{8(T_2 - T_1)}$$
11

Where L_1 = initial distance (m) at time T_1 between the drogue pairs L_2 = distance between the drogue pairs at time T_2 T_2 - T_1 = total time in seconds.

3. MATERIAL AND METHODS

3.1 Data

I. Drogue measurements

Between 1981 and 1992, the CSIR undertook numerous studies at a number of coastal development areas to determine the feasibility of ocean outfalls as options for sewage disposal. These studies included the Lagrangian recording of currents, using surface and sub-surface drogues. The paths of the drogues were recorded by accurate position fixing in time and space. Drogue data were collected from the following areas:

- Noordwesbaai (north of Saldanha Bay). Representing a 'rugged' west coast coastline, subjected to typical southerly and north-westerly wind conditions:
- 2. False Bay. The bay is relatively calm with regard to wave conditions but the northern part of the bay is exposed to the strong south-easterly and north-westerly conditions. Ambient currents are relatively weak, averaging less than 0,2 m s⁻¹;
- 3. Hout Bay. A semi-enclosed bay exposed to the south-easterly and northerly wind conditions, but the wind fields are not homogeneous and varied continuously due to the complex coastline configuration and surrounding mountain areas;
- 4. Vlees Bay. Studies were undertaken at Vlees Bay, south-west of Mossel Bay. This bay is not sheltered against waves or from the summer and winter wind conditions. Ambient currents are subjected to the west flowing Agulhas Current and anti-clockwise eddies.
- East London. A 'straight' coastline subjected to strong ambient longshore currents (Agulhas) and strong south-westerly and north-easterly winds which together with the ambient currents result in a dynamic, turbulent environment.

Data from each study site, including surface and sub-surface drogues, were assessed to select suitable data sets. Suitable data were defined as those where drogue pairs were released and retrieved more or less simultaneously. Data sets were evaluated and qualified for the estimation of the diffusion coefficients according to equation 8. The selected data for each site are provided in Appendix A.

ii. Tracer (Rhodamine-B) measurements

Between 1991 and 1993 initial dilution performance tests were conducted at three deep sea outfalls along the South African coastline (WRC, 1994), i.e Richards Bay (Mhlathuze Water A-line), Vlees Bay (Mossgas discharge into Vlees Bay) and Hout Bay (sewage outfall).

The aim of this study was to determine the actual achievable initial dilutions for comparison to theories generally applied in South Africa. During these field exercises the concentrations in the moving waste field were also recorded as accurately as possible, although a review of the secondary dilutions was not part of the scope of the project. The raw data for Richards Bay, Vlees Bay and Hout Bay is presented in Appendix A. The data parameters are time, concentration and distance from diffuser.

1. Richards Bay. During 1980 two marine outfalls (A and B-line) were constructed at Richards Bay for the discharge of various industrial effluents and domestic sewage. The location of the outfalls are illustrated in Figure 1a. The entire outfall scheme is managed by the Mhlathuze Water.

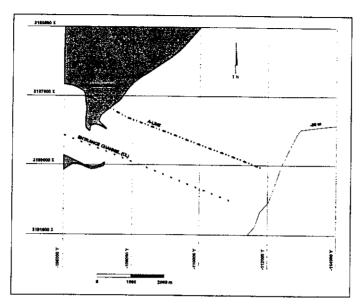


Figure 1a. Richards Bay: Locality map of sea outfall (A-line)

The A-line, which discharges in 29 m water depth approximately 4,8 km offshore, was designed to accommodate buoyant effluents from various industries and the domestic sewage from Richards Bay.

Another pipeline, the B-line discharges in 24 m water depth and was designed for discharging 'dense' effluents from the fertilizer factory (presently owned by Indian Ocean Fertilizers).

The main features and specifications of the Richards Bay marine outfall (A-line) which has been in operation since 1984 are:

Effluent type: Industrial and domestic (buoyant in

comparison with seawater)

Discharge pattern: Continuous

Material: HDPE (High density polyethylene)

~630 m

Pipeline length: 4,8 km offshore

Water depth: 29 m

Diffuser length:

Internal diameter: 920 mm

Port configuration: 69x75 mm Ø, 6x115 mm Ø and 21x75 mm Ø

• Port spacing: 6,5 m

Design flow rate: 160 000 m³ day⁻¹ (1,85 m³ s⁻¹)

The following data refer to the conditions on the day of the field experiment:

Flow:

Constant at 1,35 m³ s⁻¹

Effluent density:

1 010,1 g l⁻¹

Diffuser:

96 ports discharged without obstruction. However, measurements refer only to the 6 x 115 mm Ø ports which, for the estimation of secondary dilutions, represented a diffuser with a length of 32,5 m.

Currents:

The surface and sub-surface currents were as

follows:

Surface 0,47 - 0,63	DIRECTION (degrees to N)
1 -1 0100	1 202 - 225 1
-5 m 0,34 - 0,48	202 - 225
-10 m 0,32 - 0,38	202 - 225

The current patterns and velocity vectors are illustrated in Figure 1b.

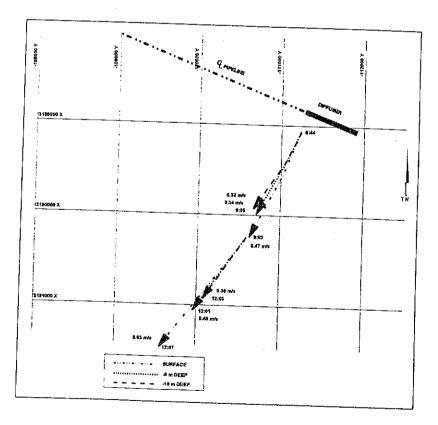


Figure 1b. Richards Bay: Currents measured on 11 September 1991

Viees Bay. This ocean outfall at Viees Bay, approximately 10 km west of 2. Mossel Bay, was constructed in 1991 for discharging effluent from the Mossgas refinery. This outfall is owned and operated by Mossgas. The location of the outfall is illustrated in Figure 2a.

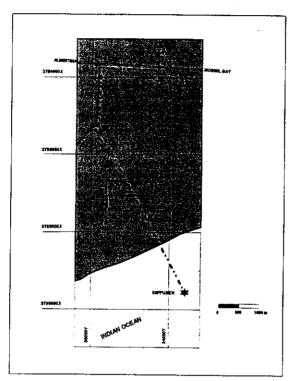


Figure 2a. Vlees Bay: Locality map of the sea outfall

The main features and specifications of the Mossgas pipeline at Vlees Bay which has been in operation since 1992 are as follows:

•	Effluent type:	Industrial (buoyant)
•	Discharge pattern:	Intermittent
•	Material:	Steel
•	Pipeline length:	9 km (refinery to diffuser)
		Diffuser is 1,4 km offshore
•	Water depth:	27 m
•	Internal diameter:	203,2 mm
•	Diffuser length:	50 m
•	Port configuration:	4 x 75 mm Ø and 1 x 79 mm Ø
•	Port spacing:	3 x 10 and 1 x 8,5 m

Design flow rates in phases:

 $0.052 \text{ m}^3 \text{ s}^{-1}$

0,136 m³ s⁻¹

0,167 m³s⁻¹

The following data refer to the conditions on the day of the field exercise:

Flow:

Constant at 0,08 m s⁻¹

Effluent density:

1 000 g J⁻¹

Diffuser:

All 5 ports were discharging without obstruction

Currents:

The surface and sub-surface currents were as

follows:

and the second state of th	SPEED (m s ⁻¹)	DIRECTION (degrees to N)
Surface	0,11 - 0,18	315 - 0
-5 m	0,07 - 0,18	317 - 315
-10 m	0,06 - 0,10	337 - 315

The current patterns and velocity vectors are illustrated in Figure 2b.

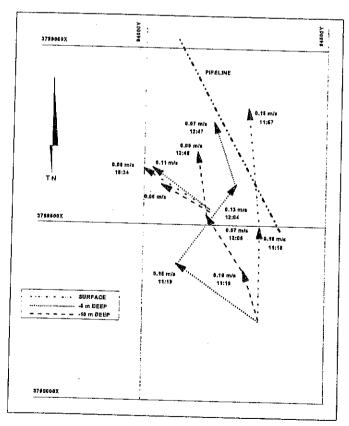


Figure 2b. Viees Bay: Currents measured on 10 November 1992

3. Hout Bay. The pipeline is owned by the Cape Metropolitan Council (formerly the Western Cape Regional Services Council). The location of the outfall is shown in Figure 3a. For discharging of the pre-treated effluent from the treatment works to the sea, the system operates on gravity flow but uses booster pumps to provide the additional pressure head when required.

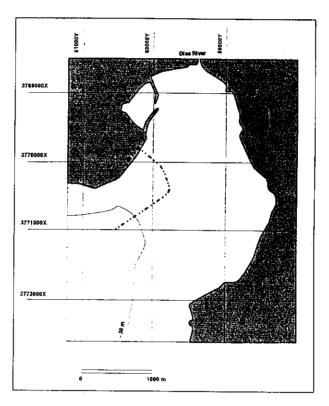


Figure 3a. Hout Bay: Locality map of the sea outfall

The main features and specifications of the marine outfall at Hout Bay which has been in operation since October 1993 are as follows:

• Effluent type: Domestic and industrial (Fishing industry)

Pre-treatment: Maceration and fine screening

Discharge pattern: Intermittent

Material: High density polyethylene (HDPE)

Pipeline length: 1,8 km offshore

Water depth: 37 m
Internal diameter: 364 mm

• Diffuser length: 114 m

Port configuration: 10 x 110 mm Ø and 5 x 140 mm Ø

Port spacing: 10 m

Maximum design flow rate: 250 m³ s⁻¹

• Flow rate in 1993: 125 m³ s⁻¹

The following data refer to the conditions on the day of the field experiment:

Flow:

Constant at 0,123 m s⁻¹

Effluent density:

1 000 g l⁻¹

Currents:

The surface and sub-surface currents were as

follows:

	SPEED (m s ⁻¹)	DIRECTION (degrees to N)
Surface	0,02 - 0,16	344 - 66
-5 m	0,02 - 0,08	152
-10 m	0,02 - 0,11	354 - 89

The current patterns and velocity vectors are illustrated in Figure 3b.

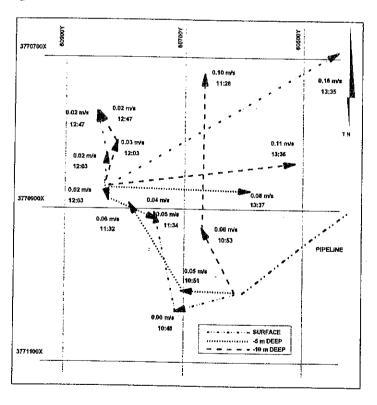


Figure 3b. Hout Bay: Currents measured on 12 October 1993

3.2 Methods

I. Determination of predicted and measured secondary dilutions

The tracer measurements at Richards Bay, Vlees Bay and Hout Bay were used to determine actual secondary dilutions. For each test the secondary dilution (S_e) was determined from C_x/C_o . The initial concentration (C_o) was determined from the measured surface concentrations in the initial surface plume (referred to as the 'boil'), after the initial dilution process. Because the previous project was designed to assess initial dilutions, the data had to be re-organized to obtain the required information for the assessment of secondary dilutions.

For each location a few groups of data points, at a certain time and distance from the release point, were selected to represent the concentrations in the moving waste field. Due to extremely weak currents together with varying directions during the Hout Bay exercise, a meaningful relationship between the concentrations and dilutions versus distance could not be obtained. The reduction in concentrations of the growing waste field can be estimated better in relation to time.

For the prediction of achievable secondary dilutions the method suggested by Brooks (1960) was used (refer to Section 2). Presently this method is generally used in the assessment of secondary dilutions for deep sea outfalls in South Africa. For South African conditions, the controlling parameter, i.e. the diffusion coefficient, is considered to be proportional to $L^{4/3}$, referred to as the '4/3 law' (generally used for 'open ocean conditions') and a α -value of 0,0005 m^{3/2} s⁻¹.

ii. Determination of prototype diffusion coefficients

To determine prototype diffusion coefficients and, subsequently α -values, from the drogue measurements Equation 11 (refer to Section 2) was used, i.e.:

$$K = \frac{{L_2}^2 - {L_1}^2}{8(T_2 - T_1)}$$

Where L_1 = initial distance (m) at time T_1 between the drogue pairs L_2 = distance between the drogue pairs at time T_2

 $T_2 - T_1 = total time in seconds.$

The α -values were subsequently determined, applying the '4/3 law', presently considered to be applicable to all 'deep sea outfall' studies (refer to Equation 4 in Section 2), i.e.:

$$K_0 = \alpha L^{4/3}$$

45

From the tracer measurements at Richards Bay, Vlees Bay and Hout Bay the actual prototype diffusion coefficient and, subsequently the α -values were determined from the actual secondary dilutions, assuming the Brooks (1960) method for 'open sea conditions', i.e. the '4/3 law', applies.

4. RESULTS AND DISCUSSION

4.1 Comparison of Predicted Secondary Dilutions to Measured Field Data

For the purpose of this project, the actual secondary dilutions measured during three field experiments are compared with predicted secondary dilutions, based on the standard method applied in South Africa, i.e. Brooks method (1960) assuming the '4/3 law' and a α -value of 0,0005 m^{2/3} s⁻¹ (Brooks, 1960; Yudelson, 1967; Okubo, 1974; Pearce, 1969).

4.1.1 Richards Bay

The surface concentrations of the tracer and secondary dilutions (log scale) versus distance measured at Richards Bay during the field experiment on 11 September 1991 are illustrated in Figures 4a and 4b, respectively.

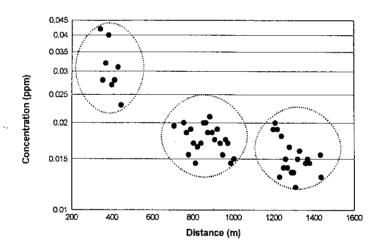


Figure 4a. Surface tracer concentrations (log scale) versus distance from discharge location for Richards Bay on 11 September 1991

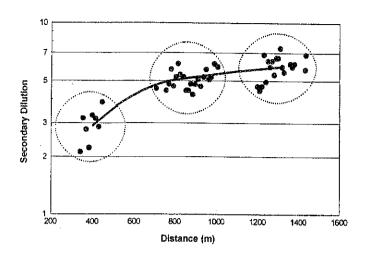


Figure 4b. Secondary dilutions (log scale) versus distance from discharge location for Richards Bay on 11 September 1991

The measured secondary dilutions (averages of grouped data) versus distance measured for Richards Bay are also shown in Table 1a.

TABLE 1a. Measured secondary dilutions versus distance measured for Richards Bay

DISTANCE (m)	SECONDARY DILUTION
400	2,98
900	5,02
1 300	5,87

To obtain a spatial overview of the behaviour of the moving waste field (transport and 'growth'), circumnavigations of the waste field were conducted (Figure 4c). Due to the strong currents on the day of the exercise, the longitudinal dispersion of the waste field was more than five times the lateral dispersion. For comparison with the theoretical predicted waste field width, using Brooks method (1960), the diameter of the 'circle' representing the measured width of the waste field was used as a width. The time it took to circumnavigate the waste field was also taken into account. These estimated waste field widths are tabulated in Table 1b.

TABLE 1b. Measured waste field width versus distance for Richards Bay

DISTANCE (m)	WIDTH (m)
500	168
1 000	185
1 600	209
2 400	280

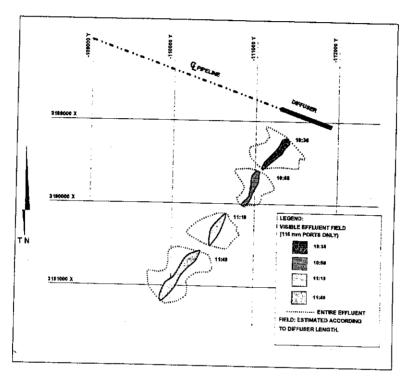


Figure 4c. Richards Bay: Transport of the waste field on 11 September 1991 (taken from WRC [1994])

To predict secondary dilutions for Richards Bay, the following input data were used (refer to Section 3 and WRC (1994)):

The initial surface waste field (b), also referred to as the 'boil', was 30 m. It relates to the diffuser width (38,5 m, i.e. 6 ports at 6,5 m spacing) and the orientation of the diffuser to the current direction which was 80 degrees on the day of the exercise;

Current speeds: Surface = 0.55 m s^{-1} -5 m = 0.41 m s^{-1} -10 m = 0.35 m s^{-1} The predicted secondary dilutions and predicted width of the waste field (according to Brooks (1960) method, assuming the '4/3 law' and a α -value of 0,0005 m^{2/3} s⁻¹) versus distance for Richards Bay are provided in Tables 1c and 1d, respectively.

TABLE 1c. Predicted secondary dilutions versus distance for Richards Bay

Մ _a (m s ⁻¹)	U _a n	DISTANCE - m							
,,		250	500	750	1 000	1 250	1 500		
0,55	4/3	1,04	1,21	1,43	1,68	1,95	2,23		
	1	1,03	1,18	1,36	1,56	1,75	1,95		
0,41	4/3	1,08	1,36	1,70	2,06	2,44	2,86		
	1	1,07	1,30	1,56	1,83	2,10	2,35		
0,35	4/3	1,12	1,48	1,89	2,33	2,86	3,36		
	1	1,11	1,39	1,70	2,03	2,33	2,64		

TABLE 1d. Predicted waste field width (L_x) versus distance for Richards Bay

(m s ⁻¹)		DISTANCE - m							
		250	500	750	1 000	1 250	1 500	2 000	
0,55	4/3	39	48	59	70	81	93	119	
	1	38	47	56	64	73	81		
0,41	4/3	42	55	70	86	102	120	158	
W. San Land	1	41	53	64	76	87	98		
0,35	4/3	44	60	78	97	117	139	185	
	1	43	57	70	83	97	110	100	

A comparison of the predicted data and the field data for both secondary dilution and waste field width versus distance from the discharge location for Richards Bay are presented in Figures 4d and 4e, respectively.

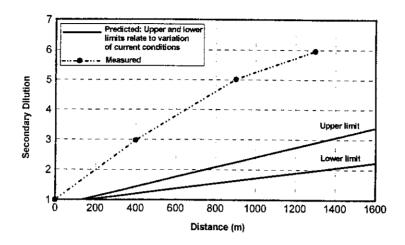


Figure 4d. Predicted and measured secondary dilutions for Richards Bay

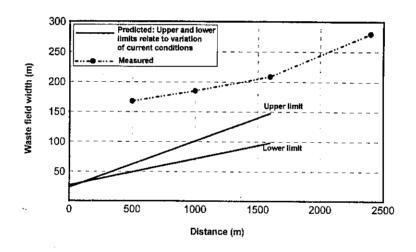


Figure 4e. Predicted and measured waste field width for Richards Bay

Using a α -value of 0,0005 m^{2/3} s⁻¹, the predicted secondary dilutions at Richards Bay were about 2,5 times less than the measured dilutions. Previous studies in the area (Pearce, 1969), had also indicated that a α -value of 0,0005 m^{2/3} s⁻¹ can be considered as conservative. The actual α -value on the day of the exercise was 0,0017 m^{2/3} s⁻¹ applying the '4/3 law'. The predicted waste field width was also about two times less than the measured width.

4.1.2 Viees Bay

The surface concentrations of the tracer and secondary dilutions (log scale) versus distance measured at Vlees Bay during the field experiment on 10 November 1992 are illustrated in Figures 5a and 5b, respectively.

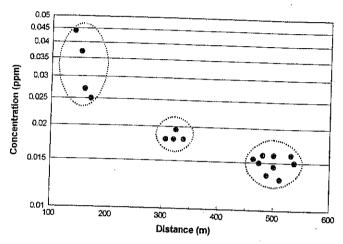


Figure 5a. Surface tracer concentrations (log scale) versus distance from discharge point for Vlees Bay on 10 November 1992

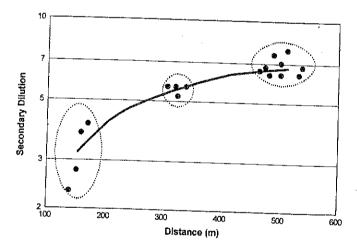


Figure 5b. Secondary dilutions (log scale) versus distance from discharge point for Vlees Bay on 10 November 1992

The measured secondary dilutions versus distance measured for Vlees Bay are also provided in Table 2a.

TABLE 2a. Measured secondary dilutions versus distance measured for Viees Bay

DISTANCE (m)	SECONDARY DILUTION
150	3,2
320	5,6
500	6,7

To obtain a spatial overview of the behaviour of the moving waste field (transport and 'growth'), circumnavigations of the waste field were conducted (Figure 5c). From these results the waste field widths were determined. The results are tabulated in Table 2b.

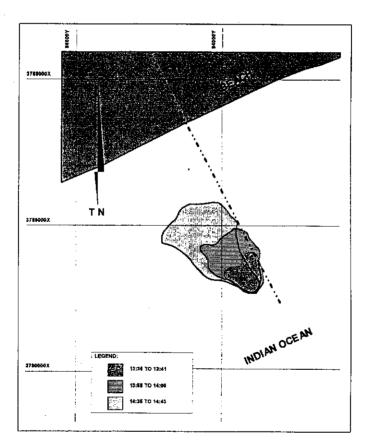


Figure 5c. Viees Bay: Transport of the waste field on 10 November 1992

TABLE 2b. Waste field width versus distance measured for Vlees Bay

DISTANCE (m)	WIDTH (m)
200	220
360	340
630	450

To predict secondary dilutions for Vlees Bay, the following input data were used (refer to Section 3 and WRC (1994)):

The initial surface waste field width (b) was 25 m, taking the diffuser length as 38,5 m and the angle between the diffuser orientation and the current direction as 40 degrees;

The predicted secondary dilutions and the predicted waste field width versus distance for Vlees Bay are provided in Tables 2c and 2d, respectively.

TABLE 2c. Predicted secondary dilutions versus distance for VIees Bay

Ս _a (m s ⁻¹)	n	DISTANCE - m				
(0)	i L	200	400	600		
0,14	4/3	1,6	2,7	3,8		
	1	1,5	2,2	2,9		
0,13	4/3	1,6	2,8	4,1		
	1	1,5	2,3	3,0		
0,08	4/3	2,3	4,5	6,9		
	1	2,0	3,3	4,5		

TABLE 2d. Predicted waste field width (L_x) versus distance for Vlees Bay

U _a (m s ^{.1})	n	DISTANCE - m				
(m s ⁻¹)		200	400	600		
0,14	4/3	54	89	130		
	1	50	75	100		
0,13	4/3	56	95	141		
	1	52	79	106		
0,08	4/3	80	153	240		
	1	69	113	157		

A comparison of the measured data and the field data for both secondary dilution and plume width versus distance from the discharge location for Vlees Bay are presented in Figures 5d and 5e, respectively.

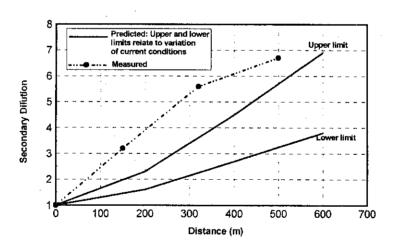


Figure 5d. Predicted and measured secondary dilutions for Vlees Bay

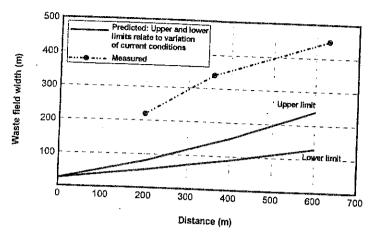


Figure 5e. Predicted and measured waste field widths for Viees Bay

For the experiment at Vlees Bay, the predicted secondary dilutions, using a α -value of 0,0005 m^{2/3} s⁻¹, were slightly less than the measured secondary dilutions.

The predicted waste field width was also about two times less than the measured width, possibly associated with the estimation of the angle between the current direction and the diffuser orientation (i.e. θ). This angle is used to calculate the effective diffuser width (i.e. the initial waste field width, b, where b = w sin θ). The estimation of the effective diffuser width also affects the prediction of secondary dilutions, but not as drastically as for the prediction of waste field width. For a relatively short diffuser, the effective diffuser width would be less affected by an inclined current considering the width of the initial waste field ('boil'), relative to the length of the 'boil'.

4.1.3 Hout Bay

Due to the varying near-stagnant conditions at Hout Bay (varying wind and near stagnant ambient currents resulted in a complex transport path) the secondary dilutions were determined versus time after surfacing of the plume. The surface concentrations of the tracer and secondary dilutions (log scale) versus time measured at Hout Bay during the field experiment on 12 October 1993 are illustrated in Figures 6a and 6b, respectively.

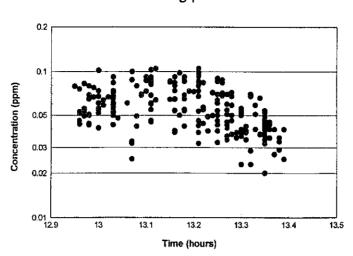


Figure 6a. Surface tracer concentrations (log scale) versus time from discharge location for Hout Bay on 12 October 1993

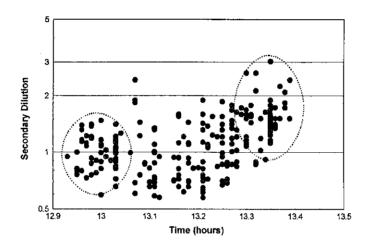


Figure 6b. Secondary dilutions (log scale) versus time from discharge location for Hout Bay on 12 October 1993

The measured secondary dilutions versus time measured for Hout Bay are also provided in Table 3a.

TABLE 3a. Measured secondary dilutions versus time measured for Hout Bay

TIME (min)	SECONDARY DILUTION
6	1
36	1.5

To obtain a spatial overview of the moving waste field (transport and 'growth'), circumnavigations of the waste field were conducted (Figure 6c). The results are tabulated in Table 3b.

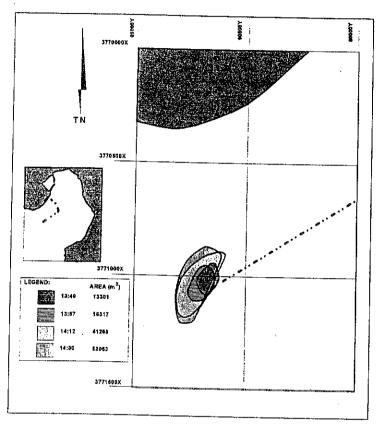


Figure 6c. Hout Bay: Transport of the waste field on 12 October 1993

TABLE 3b. Waste field width versus time measured for Hout Bay

ен. 59)

TIME (min)	WIDTH (m)		
58	130		
75	140		
90	220		
108	260		

To predict secondary dilutions for Hout Bay, the following input data were used (refer to Section 3 and WRC (1994)):

The initial waste field width (b) was 40 m - only five ports were in operation and the angle between the diffuser orientation and the current was 90 degrees;

Average current speeds: Surface = 0.09 m s^{-1} -5 m = 0.04 m s^{-1}

 $-10 \text{ m} = 0.06 \text{ m s}^{-1}$

The predicted secondary and waste field width versus time for Hout Bay are provided in Table 3c. The dilutions (based on measured concentrations) and waste field width could not be given in distance, due to the lack of a descriptive path owing to varying current conditions. After two hours the total distance was only about 500 m. The effective distance of the waste field width from the discharge location after that time was even less due to the change in currents. For the offshore environment such conditions can be considered as 'almost stagnant'.

TABLE 3c. Predicted secondary dilutions (S_e) and waste field width (L_x) versus time for Hout Bay

	n			Tì	ME (minute	es)		
		10	20	40	60	80	100	120
S,	4/3	1,05	1,25	1,78	2,46	3,16	3,94	4,74
	1	1,04	1,21	1,63	2,06	2,51	2,96	3,37
L _x (m)	4/3	53	67	98	134	172	213	257
	1	52	65	89	114	138	165	148

A comparison of the measured data and the field data for both secondary dilution and plume width versus time for Hout Bay are presented in Figures 6d and 6e, respectively.

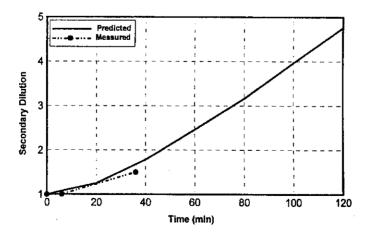


Figure 6d. Predicted and measured secondary dilutions for Hout Bay



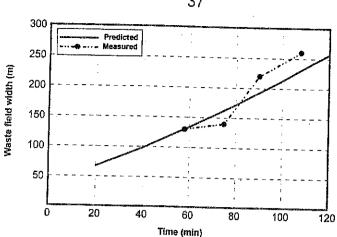


Figure 6e. Predicted and measured waste field widths for Hout Bay

Changing current conditions, which caused reversal of the moving waste field, limited the time period during which concentration measurements after the initial dilution process (i.e. after surfacing of the plume) could be conducted at Hout Bay. Due to the 'near stagnant' conditions, measured secondary dilutions were only limited to about 1,5 after 36 minutes, which were slightly less than the predicted secondary dilutions.

The relationship between the achievable secondary dilutions ($S_e = C_0/C_{max}$) and the dimensionless parameter $\beta x/b$ for n=0, n=1 and n=4/3 are illustrated in Figure 7. The measured secondary dilutions at Richards Bay, Vlees Bay and Hout Bay, using a α of $0.0005~m^{2/3}~s^{-1}$, are also super-imposed on these.

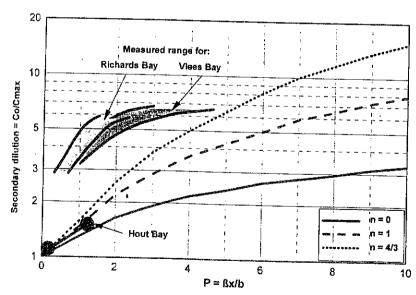


Figure 7. Relationship between secondary dilution and the dimensionless parameter (p), as well as measured secondary dilutions for Richards Bay, Vlees Bay and Hout Bay

The under-prediction of achievable secondary dilutions for Richards Bay and Vlees Bay, using a α -value of 0,0005 m^{2/3} s⁻¹, is clearly illustrated.

4.2 Determination of Prototype Diffusion Coefficients (and α -values)

A comparison of the measured and predicted secondary dilutions (based on the standard procedure applied in South Africa, i.e. method of Brooks (1960) assuming the '4/3 law' and a constant α -value of 0,0005 m^{2/3}s⁻¹) yielded a trend from under-estimation on the east coast (Richards Bay) to an over-estimation of secondary dilutions at Hout Bay in the western Cape (refer to Section 4.1) . This could imply that either the '4/3 law', i.e. assuming open ocean conditions, does not apply for all areas along the 3 000 km coastline of South Africa or that the selected α -value are site-related, especially when considering the diversity of conditions along the South African coast.

In this section an attempt will be made to determine prototype α -values (based on actual diffusion coefficient measurements) for a number of areas around the South African coastline, using available data collected along the coast over a period of 10 years (Section 3.1). As it is presently assumed that the 'open sea' condition applies to all deep sea outfalls along the South African coast, Brooks' (1960) method, assuming 'the 4/3 law' was applied in order to estimate α -values from measured diffusion coefficients.

4.2.1 Determination of diffusion coefficients (and α -values) based on drogue measurements

Extensive drogue data from a number of areas along the South African coast were used in this study, i.e. Noordwesbaai, False Bay, Hout Bay, Vlees Bay and East London. The detailed drogue data are presented in Appendix A.

From the selected drogue measurements of each study site the following parameters were calculated to determine the diffusion coefficient according to Equation 11 (refer to Section 2):

- I. Initial separation (L₁)
- ii. Separation at retrieval (L₂)
- iii. Time (T_1) for L_1

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iv. Time (T_2) for L_2
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v. Total distance (S)

vi. Total time $(T_2 - T_1)$

vii. Transport velocity: S

viii. Diffusion coefficient: $K = \frac{L_2^2 - L_1^2}{8(T_2 - T_1)}$

ix. The α -value based on the '4/3 law', i.e K = $\alpha L^{4/3}$.

The detailed sets of K-values and α -values calculated for each site are presented in Appendix A.

To gain a better understanding of the diffusion processes and the influencing parameters, an attempt was made to obtain a relationship between the estimated α -values and length scale (L_x), transport distance, current velocity, water depth and wind speed. The best fit to the raw data proved to be exponential and yielded the following relationships:

$$\begin{array}{lll} \alpha & = & 0,00038 \ e^{2,0496u} \\ \alpha & = & 0,00052 e^{0S} = 0,00052 \\ \alpha_{surface} & = & 0,00056 e^{0S} = 0,00056 \\ \alpha_{-5m} & = & 0,0005 \ e^{0S} = 0,0005 \\ \alpha & = & 0,00069 \ e^{-0,0049L} \\ \alpha & = & 0,00063 \ e^{-0,0033V} \\ \alpha & = & 0,00054 \ e^{-0,0216T} \end{array}$$

Where

u = current velocity (m s⁻¹)
S = total distance (m)
L = length scale (initial distance between drogues)
V = wind speed (knots)
T = total time (hours)

The relationship versus transport distance (S) yields a constant value of 0,00052 $m^{2/3}s^{-1}$ for α -values.

The data for each study site and for the overall data set (all sites) were grouped in various combinations of current velocities, water depth, total transport distance, length scales and wind conditions in order to determine the effect of these parameters on the estimated α -values. Detailed analysis and plots are presented in Appendix B.

The combinations were as follows:

PARAMETER	COMBINATION
Current velocities	< 0.1; 0,1 to 0,2; > 0,2 m s ⁻¹
Depth	surface and -5 m
Total distance (S)	< 500; 500 - 1 000; > 1 000 m
Length scale (L)	< 40; 40 - 80; 80 - 120 m
Wind speed	< 5; 5-15; > 15 knots
Time	<1; 1-2; >2 hours

After a statistical review of the distributions of the selected sets of grouped data (see Appendix C), the *median* value was shown to be more realistic than the average values for K- and α -values, due to the variation in magnitude and limited number of samples for certain sites.

The median K- and α -values for the various combinations are presented in Tables 4a to 4f and illustrated in Figures 8a to 8f. The notation 'S' and 'SS' relates to surface and sub-surface (-5 m) drogue measurements

TABLE 4a. Median K- and α -values for Noordwesbaai determined for various combinations of conditions

SAMPLE		CONE	MEDIAN				
#	Wind	Currents (m s ⁻¹)	S/SS	L (m)	S (m)	K-Value	α-Value
147	ALL	ALL	S/SS	ALL	ALL	0,0667	0,000375
79	ALL	ALL	S	ALL	ALL	0,073	0,00039
67	ALL	ALL	SS	ALL	ALL	0,058	0,00036

TABLE 4b. Median K- and α -values for False Bay determined for various combinations of conditions

SAMPLE #		COND		MEDIAN			
77	Wind	Currents (m s ⁻¹)	S/SS	L (m)	S (m)	K-Value	α-Value
51	ALL	ALL	S/SS	ALL	ALL	0.1252	
20	ALL	ALL	S	ALL	ALL		0,00037
31	ALL	ALL	SS			0,1248	0,000317
31	ALL	ALL	SS	ALL	ALL	0,1256	0,00

TABLE 4c. Median K- and α -values for Hout Bay determined for various combinations of conditions

SAMPLE #		COND	MEDIAN		DIAN		
11°	Wind	Currents (m s ⁻¹)	S/SS	L (m)	S (m)	K-Value	α-Value
57	ALL	ALL	S/SS	ALL	ALL	0,112	
27	ALL	ALL	S	ALL	ALL		0,00077
28	ALL	ALL				0,227	0,00091
	· >		SS	ALL	ALL	0,0809	0,00074

TABLE 4d. Median K- and α -values for VIees Bay determined for various combinations of conditions

SAMPLE #		COND		MEDIAN			
**	Wind	Currents (m s ⁻¹)	S/SS	L (m)	S (m)	K-Value	α-Value
54	ALL	ALL	S/SS	ALL	ALL	0,1632	
24	ALL	ALL	S	ALL	ALL		0,00051
30	Al I	All			/\L	0,2064	0,00046
	, , , , , , , , , , , , , , , , , , , ,	/\LL	SS	ALL	ALL	0,1438	0,00052

TABLE 4e. Median K- and α -values for East London determined for various combinations of conditions

SAMPLE		COND	MEL	MEDIAN			
#	Wind	Currents (m s ⁻¹)	S/SS	L (m)	S (m)	K-Value	α-Value
93	ALL	ALL	S/SS	ALL	ALL	0,1564	0,0011
52	ALL	ALL	S	ALL	ALL	0,1681	0,00112
41	ALL	ALL	SS	ALL	ALL	0,1304	0,00109

TABLE 4f. Median K- and α-values for all sites (grouped) determined for various combinations of conditions

SAMPLE			CONDI	TIONS			MEI	DIAN
#	Wind	Currents (m s ⁻¹)	Time (hours)	S/SS	L (m)	S (m)	K-Value	α-Value
400	ALL	ALL	ALL	S/SS	ALL	ALL	0.1050	0,00058
205	ALL	ALL	ALL	S	ALL	ALL	0,1197	0,00062
199	ALL	ALL	ALL	SS	ALL	ALL	0,0931	0,00051
158	ALL	< 0,1	ALL	S/SS	ALL	ALL	0,0841	0,00046
137	ALL	0,1-0,2	ALL	S/SS	ALL	ALL	0,1188	0,00054
109	ALL	> 0,2	ALL	S/SS	ALL	ALL	0,1416	0,00083
159	ALL	ALL	ALL	S/SS	ALL	< 500	0,1042	0,00055
114	ALL	ALL	ALL	S/SS	ALL	500-1000	0,0856	0,00050
131	ALL	ALL	ALL	S/SS	ALL.	> 1000	0,1263	0,00062
136	ALL	ALL	ALL	S/SS	< 40	ALL	0,0547	0,00079
192	ALL	ALL	ALL	S/SS	40-80	ALL	0,1220	0,00054
76	ALL	ALL	ALL	S/SS	> 80	ALL	0,1782	0,000361
70	< 5	ALL	ALL	S/SS	ALL	ALL	0,0865	0,000727
193	5-15	ALL	ALL	S/SS	ALL	ALL	0,1252	0,00060
73	>15	ALL	ALL	S/SS	ALL	ALL	0,2104	0,00074
133	ALL	ALL	< 1	S/SS	ALL	ALL	0,1429	0,00082
146	ALL	ALL	1-2	S/SS	ALL	ALL	0,0931	0,00051
120	ALL	ALL	> 2	S/SS	ALL	ALL	0,0793	0,00045

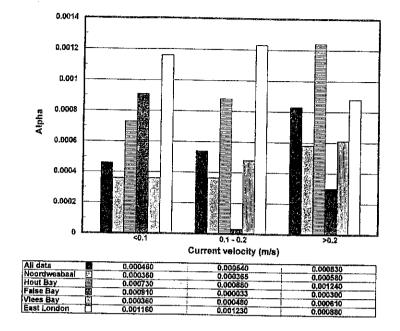
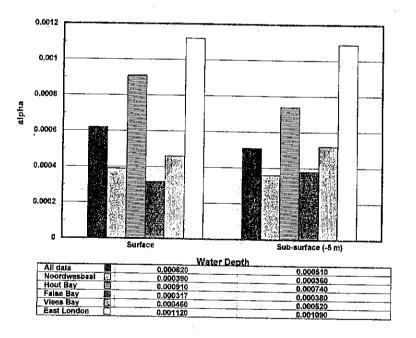


Figure 8a. Median of α -values calculated for each sites, in both surface and sub-surface (-5 m) waters at different current velocities



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Figure 8b. Median of α -values calculated for all sites, in both surface and subsurface (-5 m) waters at different water depths

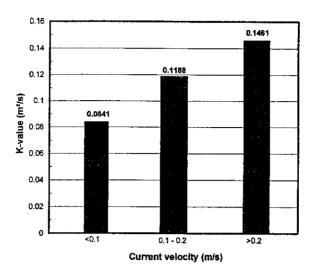


Figure 8c. Median of K-values calculated for all sites, in both surface and subsurface (-5 m) waters at different current velocities

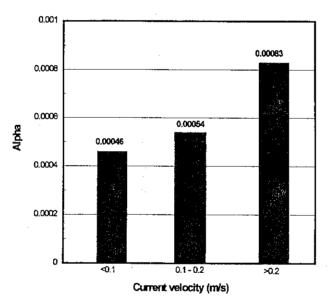


Figure 8d. Median of α -values calculated for all sites, in both surface and subsurface (-5 m) waters at different current velocities

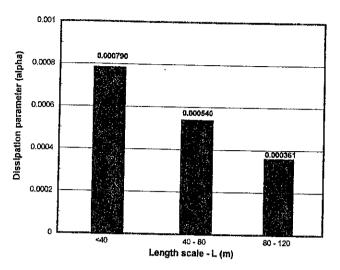


Figure 8e. Median of α-values calculated for all sites, in both surface and subsurface (-5 m) waters at different length scales

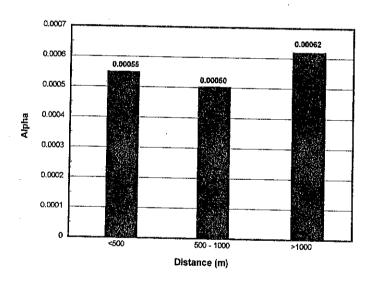


Figure 8f. Median of α -values calculated for all sites, in both surface and subsurface (-5 m) waters at different distances

The median and exponential fit versus distance of all the $\,$ K- and α -values calculated from the selected drogue data sets presented in Table 5.

TABLE 5. The median and exponential fit versus distance of all the K- and α -values calculated from the selected drogue data sets

LOCATION	K-V	ALUE	α-VALUE		
	Median	Exp-fit	Median	Exp-fit	
All sites	0,105	0,0948	0,00058	0,00052	
Noordwesbaai	0,0667	0,05228	0,000375	0,00031	
Hout Bay	0,1120	0,1236	0,00077	0,00088	
False Bay	0,1252	0,1366	0,00037	0,0005	
Vlees Bay	0,1632	0,191	0,00051	0,00057	
East London	0,1563	0,166	0,0011	0,00103	

Although the median α -values of 0,00058 m^{2/3} s⁻¹ for all sites compare well with the general suggested value of 0,0005 m^{2/3} s⁻¹ for open sea conditions, the individual values for certain areas, for example Noordwesbaai differ substantially from the general α -value of 0,0005 m^{2/3} s⁻¹.

The effect of current velocity on the turbulent behaviour of the receiving water and subsequent α -value is illustrated in Figure 8b, where the α -value for currents less than 0,1 m s⁻¹ is 0,00046 m^{2/3} s⁻¹ compared to 0,00083 m^{2/3} s⁻¹ for currents greater than 0,2 m s⁻¹. The relation between the α -value and current speed for all available drogue data is:

$$\alpha = 0.00038 e^{2.0496u}$$

In sheltered areas, where currents are weaker, a value of less than $0.0005 \mathrm{m}^{2/3}~\mathrm{s}^{-1}$ should be used for a conservative approach, whereas in a more dynamic environment, for example east coast conditions (Richards Bay), values greater than $0.0005 \mathrm{m}^{2/3}~\mathrm{s}^{-1}$ can be applied.

The validity of the equation $K = \alpha L^n$ which is independent of the transport distance was confirmed. The difference between the α -values for total distances of less than 500 m and greater than 1 000 m is less than 0,0001 m^{2/3} s⁻¹. The exponential fit yielded a constant:

$$\alpha = 0,00052$$

The estimated median α -value for sub-surface currents (-5 m water depth) is approximately 20 percent lower than the estimated value for surface currents. This is important especially when conducting performance evaluations of deep sea outfalls by the measuring of actual dilutions using tracer material in an effluent, as most of the samples are taken close to the sea surface.

The influence of the length scale (L_x) obtained from the drogue data is illustrated in Figure 8e. For a length scale of between 80 and 120 m the mean α -value (0,000361 m^{2/3} s⁻¹) is less than two times the α -value for a length scale of less than 40 m (0,00079 m^{2/3} s⁻¹).

4.2.2 Determination of diffusion coefficients (and $\alpha\text{-values})$ based on tracer measurements

As explained in Section 3, the measured concentration data had to be re-organised in 'groups' so as to obtain a meaningful description of the moving waste field with regard to the reduction of concentrations and the achievable secondary dilutions. The grouped data (concentrations and secondary dilutions) versus distance are illustrated in Figures 4a and 4b and 5a and 5b for Richards Bay and Vlees Bay, respectively and versus time for Hout Bay in Figures 6a and 6b.

The average and median initial surface concentrations (in the initial surface plume, i.e. the 'boil') and concentrations at a distance x from the 'boil' for the experiments at Richards Bay and Vlees Bay are shown in Table 6a and 6b. Due to the weak and varying current directions on the day of the Hout Bay experiment and the subsequent meandering of the waste field, the reduction in concentrations was related to time after the surfacing of the dye. The average and median concentrations in the 'boil' measured at different times are given in Table 6c.

TABLE 6a. Average and median surface concentrations in the initial surface plume and at a distance for Richards Bay

	TRACER CONCENTRATIONS (ppm)					
	INITIAL	400 m	900 m	1300 m		
AVERAGE	0,08889	0,0314	0,01766	0,01532		
MEDIAN	0,008	0,0295	0,0175	0,0150		

TABLE 6b. Average and median surface concentrations in the initial surface plume and at a distance for Vlees Bay

	TRACER CONCENTRATIONS (ppm)					
	INITIAL	150 m	320 m	500 m		
AVERAGE	0,1027	0,03325	0,0184	0,01494		
MEDIAN	0,1010	0,032	0,0180	0,0150		

TABLE 6c. Average and median surface concentrations in the initial surface plume and at specific times after release for Hout Bay

	TRACER CONCENTRATIONS (ppm)				
	INITIAL	6 min	36 min		
AVERAGE	0,0619	0,0665	0,0426		
MEDIAN	0,0605	0,0683	0,0400		

The current velocities on the days of the field measurements for the three sites are presented in Table 7.

TABLE 7. Current velocities on the days of the field measurements at the three sites

		CURRENT VELOCITY (m s ⁻¹)					
	Before the exercise		After the	exercise	Average		
	Surface	- 5 m	Surface	- 5 m	1		
Richards Bay	0,47	0,34	0,63	0,48	0,48		
Viees Bay	0,16	0,12	0,11	0,09	0,12		
Hout Bay	0,02	0,02	0,16	0,08	0,07		

The average of the surface and sub-surface (- 5 m) current speeds before and after the exercises were determined to apply to analytical estimations of the diffusion coefficients.

For the determination of the initial diffusion coefficient (K_o) and α -value applying the method of Brooks (1960), the initial length scale is taken as the width of the 'boil' in the direction of the ambient current. It was proposed that the length scale (L_x) is equal to the effective width of the diffuser (b), i.e. :

 $L = b \sin \theta$

Where θ = angle between the ambient current direction and the diffuser orientation.

For Richards Bay, Vlees Bay and Hout Bay these were estimated as follows:

LOCATION	DIFFUSER WIDTH - b (m)
Richards Bay	30*
Viees Bay	25**
Hout Bay	40***

The surface plume related only to 6 x 115 mm ports (6,5 m spacing) and an angle of 80 degrees to the current direction.

** Angle between diffuser and current direction was 40 degrees. Total length of diffuser 38,5 m.

Only five ports were in operation. Angle between diffuser and current approximately 90 degrees.

The estimated diffusion coefficients and α -value, based on the '4/3 law' according to the method of Brooks (1960), for Richards Bay and Vlees Bay at selected distances and in the case of Hout Bay, after 6 minutes and 36 minutes after the surfacing of the dye are presented in Table 8.

TABLE 8. Diffusion coefficients (K) and α -value (assuming the '4/3 law') calculated for the three sites at different distances from the discharge location

SITE	DISTANCE (m)	DILUTION	р	u (m s ⁻¹)	K/L²	α-value
Richards Bay	400	2.98	2,4	0,4	0,0002	0,00193
	900	5.02	4,1	0,48	0,00018	0,00174
	1300	5.87	4,6	0,56	0,00017	0,00164
Viees Bay	150	3.2	2,6	0,14	0,0002	0,0011
	320	5.6	4,5	0,12	0,00014	0,00077
	500	6.7	5,2	0,10	0,00009	0,0005
Hout Bay	TIME (min) 6 36	1 1,5	- 1,1	- 0,07	0,00006	0,0007

The median α -values, calculated from the tracer measurements, for Richards Bay, Vlees Bay and Hout Bay were 0,00177, 0,00079 and 0,0007 m^{2/3} s⁻¹, respectively, assuming that the '4/3 law' applied.

Although measured on one occasion only, the median α -value of 0,00177 m^{2/3} s⁻¹ at Richards Bay (as determined from tracer concentration measurements) corresponds to similar dynamic conditions such as at East London where the average α -value determined from measurements with drogue pairs yielded a median value of 0,0011 m^{2/3} s⁻¹.

For Vlees Bay, the median α -value (0,00079 m^{2/3} s¹) determined from the tracer measurements is approximately 50 percent higher than the median value (0,00051 m^{2/3} s⁻¹) calculated from the drogue measurements.

For Hout Bay the α -value of 0,0007 m^{2/3} s⁻¹ calculated from the tracer measurements, corresponds well with the median value 0,00077 m^{2/3} s⁻¹ derived from the drogue measurements.

5. CONCLUSIONS AND RECOMMENDATIONS

- Results from this project indicate that the 'general' procedure applied in the prediction of achievable secondary dilutions for deep sea outfalls along the South African coast, i.e. the method of Brooks (1960), assuming the '4/3 law' and a α -value of 0,0005 m²/3 s¹ does not always apply. This could imply that either the '4/3 law', i.e. assuming open ocean conditions, does not apply for all areas along the 3 000 km coastline of South Africa or that the selected α -value are site-related, especially when considering the diversity of conditions along the South African coast.
- 5.2 Although the median α-value of 0,0005 m^{2/3}s⁻¹, obtained for grouping all available drogue measurements, confirmed the 'general' α-value suggested by numerous authors, the α-values estimated for the individual sites, clearly showed the diversity of conditions along the 3 000 km South African coastline. However, the expected effects of current velocities, transport distance, the length scale and water depth were also confirmed by the drogue measurements.
- 5.3 For Richards Bay, the conservative prediction of secondary dilutions was most probably due to an incorrect α-values, taken that 'open sea condition', i.e. the '4/3 law' according to the method of Brooks (1960), definitely apply. However, the relatively low α-values obtained at Noordwesbaai on the west coast, may indicate that the '4/3 law' does not apply to a 'rugged' coastline (often found on the South African west coast), where the configuration of the coastline may affect the 'free spreading' (as is assumed by the '4/3 law') of the waste field.
- Based on these findings the use of a 'general' α-value is therefore not recommended in determining the secondary dilutions and subsequent impact of an effluent waste field at distant locations in South Africa. The most typical values for certain areas should be considered and if detailed field measurements cannot be obtained to establish the actual diffusion coefficients, at least a range of values should be considered. The diffusion coefficients and α-values determined in this report could be used as a rough guideline.

- Although the *prediction* of achievable secondary dilutions and subsequent impact at distant locations is considered appropriate for planning and feasibility phases of an outfall project, taking into account the near shore processes and the coastline configuration, it is strongly recommended that *field measurements* be conducted to obtain more accurate site specific diffusion characteristics of the area for the optimisation of the outfall during the detailed design phase.
- Irrespective of the procedure or method used to predict the behaviour of a moving waste field, i.e. analytical methods or numerical modelling, a thorough understanding of the physical processes, the sensitivity and limitations of the prediction method or procedure to all variables, as well as the applicability of a certain methods to a specific area or near shore conditions is essential before any quantitative conclusions can be made with regard to the detailed outfall design. A simple equation or a 'black box' approach is therefore not always considered to be sufficient in providing reliable quantitative results on achievable secondary dilutions, especially taking into account the consequences of detrimental impact in terms of ecological, health and economical aspects, both in the short and long term.

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APPENDIX A SELECTED DROGUE DATA

TABLE A.1 Drogue measurements, K-values and Alpha-values for Noordwesbaai

Aloha-value	Alpha-value 7 0.0037108156 3 0.000483345 4 0.0026772458 4 0.002176871 9 0.000322331 2 3.01241E-05 3 0.0112107017 0.000345784 0.00011815604 -2.12884E-05 0.0001183037 0.00011815604 -2.12884E-05 0.00011815604 -2.12884E-05 0.00011815604 0.00011815604 0.000181489 0.00011815604 0.0001311489 0.000184138 0.00018974138 0.0001311489 0.00018974138 0.00018974138 0.00018974138 0.0001897438 0.0001897438 0.0001897449 0.00018886884 6.32689E-05 0.0001887877	3.80327E-05 0.0042348807
K-value		0.00821759 1.91938776 C
×	9255 36541 38284 34782 38374 38374 38374 38497 38497 38497 37194 37194 37194 37194 37194 37194 37194 37194 37194 37194 37196 38372 37887 37887 37887 3789 3789	37035 37394
>-	90006 106727 106727 106710 107329 107329 107329 106704 106117 106117 106239 106334 106336 106330 106330 106419 106419 106419 106419 106419 106419 106419	107027 106966
Time-out	144548 135400 125300 130300 134400 144500 1141200 115000 1233000 133000 135600 135600 177300	152900
×	8884 38406 38623 37418 38623 38623 38623 38716 37116 37116 37716 38706 38706 38706 38706 38700 37773 37774 37870 37774 37870 37774 37870 37774 37870 37774 37870 37775 387774 37870 37775 387775 387776 377776 377776 377776 37776 37776 37776 37776 37776 37776 37776 37776 37776 37776 3	37801 38375
>-	87465 106932 106936 106917 107462 107462 107607 108924 106939 106937 108965	106940
Time-in	11342 111800 112200 112300 115800 115800 123600 104100 104100 104100 104100 104100 104100 105700 104100 105700 105700 105700 105700 105800 105	135500
×	6	37910 1
· >- -	8488668268268666	106975 3
Tlme-out	888888888888888888888888888888888888888	153400 1
×	8883 38443 38643 37415 38588 38588 38522 38109 37184 38402 38404 38402 38404 37691 37784 37892 38101 38007	38455
>	106961 106961 106964 107090 107527 106963 10683 10683 10683 10683 10683 10683 10683 10683 10696	
Time-in	113459 111800 112300 115800 115800 115800 123600 104100 104100 125100 125100 125100 145100 151100 152500 145800 16520 16520 165200 1652	-
SS/S	\$\text{S} \times \text{S} \tim	-
Date	851118 811215 811215 811216 811216 811216 811216 820209 820209 820209 820209 820209 820209 820209 820209 820209 820209 820209 820209 820301 820301 820302 820302 820302 820302 820302 820302 820302 820302 820302 820302 820302	

ie Alpha-value	333 7.41969E-05	471 0.0004079507	524 7.3734E-05	388 0.0003430954		256 0.0009133656	5 0.0007734762	131 0.0148191568			019 0.0019337002	424 0.0001674983	353 0.0007544809	216 0,0009800779	187 0.0005425486	905 0.0013053294	311 0.0005528127	74 0.0023779518	941 0.0002828583	263 9.75448E-05	071 0.0001684971	462 0.0002094287	33 0.0024289847	XX4 0,0025196693	17 3:39256E-05	66 0.000116412	788 0.0004645438	69 0.0001261372	45 6.87843E-05	19 5.05611E-05	0.0039048401	88 0.0033913751	95 0.0003856398	65 0.0009010286	22 0.0018257908	36 0.0011980442	29 0.0066907782
K-value	0.02083333	0.14018471	0.01559524	0.16334388	0.11025641	0.09285256	0.14625	12.2071131	0.48225211	0.01436012	0.51331019	0.01742424	0.30824653	0.19664216	0.09313187	0.30717905	0.19560811	0.38074074	0.04727941	0.00855263	0.02366071	0.03655462	0.24933333	0.44877604	0.00380117	0.01966766	0.10503788	0.00862069	0.03582245	0.0047619	0.41002604	1.00429688	0.11907895	0.11880165	0.12668822	0.14452736	0.92321429
×	37224	37125	38178	39121	36229	37452	38940	38782	37852	36208	36612	37574	37634	38534	38622	35631	36992	36655	37776	38165	36007	36755	35646	36334	38068	38800	35925	36506	37818	38286	37291	38327	35976	36283	37609	38424	35622
>	108956	108921	107266	107209	107104	105985	105409	106525	107144	109124	109188	108486	108569	106514	106728	108854	108630	107464	107220	107156	109444	109195	108552	108415	106280	106195	109335	109162	107530	107323	106185	106184	107445	107475	107375	107196	109797
Time-out	121600	114100	123600	124400	133600	150500	151800	160100	125500	131400	131700	105300	105100	110800	111400	140600	132500	142500	143100	133700	135200	135800	160500	161400	164200	163000	112800	112300	114200	114700	135400	134400	132700	133000	155000	152200	162400
×	37579	37411	38561	38726	36558	38130	36309	39448	38375	37212	37314	37543	37614	38501	38551	37175	37410	37604	38247	38492	37521	37561	36713	36901	38580	39083	37120	36970	38347	38475	38316	38436	36350	36842	38288	38494	37250
>	108851	108818	107099	107116	107282	105977	105434	105488	107036	108902	108872	108567	108624	106870	106906	108738	108694	107261	107101	107121	109310	109302	108184	108152	106212	106167	109112	109099	107109	107180	105798	105785	107299	107168	107151	107100	109130
Time-in	103800	104000	105100	112600	130800	135900	144800	145200	113600	114700	114900	103000	102700	94400	94500	125300	125100	133900	130600	121900	120100	115900	145100	145300	154500	150800	103200	105500	104400	104500	122000	122130	130000	123030	145130	144630	150800
×	37297	37014	38157	39106	36202	37511	38366	39433	37990	36230	36732	37603	37529	38527	38651	35746	37081	36858	37765	38170	36021	36820	35546	36233	38037	38842	35872	36517	37815	38300	37160	38121	36017	36363	37545	38364	35432
>-	108952	108897	107223	107146	107150	106006	105374	106425	107134	109146	109298	108480	108597	106618	106802	108818	108583	107435	107283	107134	109430	109182	108548	108512	106269	106160	109393	109180	107560	107300	106238	106219	107399	107467	107372	107203	109785
Time-out	114200	121400	123600	124500	133500	150400	151900	160100	125300	131200	131900	105300	105000	111100	111600	140800	132600	142400	143200	133500	135100	135900	160600	161200	164200	163200	112900	112400	114300	114800	135600	134200	132800	133100	154900	152330	162530
×	37530	37460	38537	38824	36520	38162	39347	36266	38404	37202	37361	37553	37531	38522	38571	37232	37490	37559	38291	38467	37561	37586	36685	36880	38565	39123	37070	36862	38345	38500	38336	38450	36405	36877	38265	38465	37209
>	108803	108755	107049	107088	107287	105978	105400	105464	107032	108919	108918	108536	108661	106919	106949	108719	108710	107262	107116	107107	109302	109261	108200	108196	106181	106131	109142	109108	107218	107197	105824	105855	107250	107151	107144	107078	109109
Tfme-in	103800	103700	105100	112600	130900	135900	144900	145100	113400	114800	114900	103100	102600	94600	94500	125400	124900	133900	130700	121900	115900	120000	145100	145200	154500	150800	103400	106600	10440	104500	122000	122200	125930	123030	145100	145000	150830
SISS	SS	Ø	Ø	SS	ຜ	Ø	Ø	SS	SS	ဟ	SS	ဟ	SS	Ø	SS	တ	SS	တ	SS	SS	Ø	SS	တ	SS	ဟ	SS	Ø	SS	Ø	SS	ဟ	SS	S	SS	S	SS	Ø
Date	820331	820331	820331	820331	820331	820331	820331	820331	820401	820401	820401	820402	820402	820402	820402	820805	820805	820805	820805	820806	820806	820806	820806	820806	820806	820806	820903	820903	820903	820903	820903	820903	820903	820903	820903	820903	820903

	N-Value Alpha-value 0.0460452 0.0012596822 0.0948203 0.0007007845 0.0248203 0.0007007845 0.0248111 0.0001481774 0.01965726 9.52445E-05 0.06835833 0.0003878383 0.0835823 0.0002878383 0.08358283 0.0003878383 0.18363636 0.00028444 0.03204066 0.000171752 0.0081409 -2.38834E-05 0.03204066 0.000171752 0.0081409 -2.38834E-05 0.1310559 0.000394334 0.1310559 0.0003812814 0.0641561 0.0003812814 0.07288811 0.000381281 0.08158538 0.000381289 0.0485614 0.0003810128 0.01288918 0.0005810128 0.01288918 0.0005810128 0.022802891 0.0002810128 0.022802891 0.000323331 0.08812281 0.0002810128 0.003204495 0.000328943 0.003907135 0.0002289723 </th
· >	36583 386374 37160 38887 38887 38945 39251 39251 39251 39450 39450 39450 39451 40123 37630 39450 39636
	8 8 7 7 8 4 7 8 8 8 8 8 8 8 8 8 8 8 8 8
>	109493 108883 108883 107884 107285 107288 107288 107288 107288 107288 107288 107288 107288 107288 107288 107288 107288 107288 107283 108509
Tlme-out	
×	37354 37679 38658 38626 38114 38114 38114 38211 37779 37779 37805 38857 36603 37604 37779 37805 38857 36877 36818 37700 37819 37819 37710 37819 37710 37710 37710
>-	106864 106865 106865 106871 106712 106712 106740 106725 10725 10725 1072
Time-in	
THE	151030 10540 111740 111740 90120 90130 90130 90130 90130 90130 12500 12500 12500 133804 133804 133804 133804 133804 133804 133804 13484
×	4 36585 3 36457 3 38410 3 38810 3 39454 3 36517 3 39454 3 36517 3 39454 3 37730 3 37457 3 3676 3 37457 3 3676 3 37457 3 3676 3 37457 3 3676 3 37457 3 3676 3 37457 3 3676 3 3762 3 3676 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3
>	109454 108923 1068283 107731 107833 108528 108528 107578 108975 107819 107819 107819 108534 108534 108534 108534 108534 108534 108534 108534 108534 108534 108534 108534 108534 108534 108535 108536 107577 108537 1
Time-out	
Time	161000 133200 142100 113520 115300 104420 132720 152340 152340 115020 902240 115020 902240 112000 112000 112000 112000 112000 112010 114001 114001 123944 14405 16394 14403 16394 164175 164175 164175
×	37364 3867 38159 3824 3824 3878 3878 3878 3872 3872 3872 3872 3882 37758 37758 37758 37758 37834 3882 37834 37834 37834 3882 37834 37834 37834 3882 37834 3882 37834 3882 37834 3882 37834 3882 37834 3882 37834 3882 37834 3882 37834 3882 3882 3882 3882 3882 3882 3882 3
>	108981 108981 108982 108672 107780 108688 108902 108902 108028 108028 108028 108028 108028 108028 108028 108028 108028 108028 108028 108028 108028 108028 108028 108028 108028 108039
Time-in	151100 105420 1111840 901040 90220 92140 125800 143820 900350 900350 100700 100700 100700 100700 100700 100700 100700 100700 100700 100700 100700 100700 100700 110750 110
SISS	88 88 88 88 88 88 88 88 88 88 88 88 88
Date	S S S S S S S S S S S S S S S S S S S
٥	820200 8210101 8210102 821012 821012 821012 821012 821104 821104 821208 820112 830112 830112 830125 830225 830225

Alpha-value	3 2.55682E-05	_	-	_	0.0057278439	_		_							_					Ų			0.0008978128	2.95247E-05	0.0001552216	1.75519E-05	0.0004365059	5.51907E-05	0.0003233897	0.0002912075	3.02837E-05	0.0008137103	_	0.0050809221
K-value	0.01254688	0.00883358	0.42857361	0.08963869	1.8118587	0.57112572	0.1150108	0.0659374	0.08751211	0.04587424	0.0008424	0.00131839	0.00180671	0.01065952	0.23799822	0.27517668	0.15877965	0.02059301	0.00158286	0.06397182	0.07476504	0.05639083	0.12409512	0.00508461	0.03679042	0.00240207	0.01120175	0.0042887	0.05437057	0.051517	0,00128728	0.250951	0.13685675	0.3390056
×	36577	36972	37667	37740	39213	39684	38174	36261	35988	38828	38251	36927	36434	37336	37316	37447	36802	38672	36075	37266	37305	37417	38403	39793	36557	37334	36612	38369	37820	39128	38591	38113	37917	38954
>	109103	108280	108684	109090	106748	106899	107756	108149	108195	106891	106471	106857	106820	107918	107517	109103	109533	109436	107286	107182	107881	107850	107920	107776	107713	107841	107086	108137	108701	106242	106534	108431	108966	106891
Time-out	113532	151746	104012	94137	95452	111716	133017	105227	105433	121903	133425	131219	124141	140748	141152	144048	134222	142310	133145	140727	112819	110123	140527	141422	143137	142720	135426	134037	115520	141025	140534	152621	153015	151331
×	37795	37351	37710	37701	38821	38824	38227	37406	37398	38844	38975	36951	36817	38017	37832	37591	37947	37976	38757	38828	37777	37825	36006	39146	38027	38090	36935	38065	37945	38710	38611	38127	38030	38638
>	108880	108160	108984	109049	106924	106936	107797	108870	108830	106891	106794	106987	106940	107913	107671	109052	108971	108982	107047	106951	108708	108678	107069	107053	107931	108040	107035	108769	108772	106966	106677	108698	108880	106835
Time-in	82418	124948	82617	85056	84028	83717	131601	90803	90857	114326	92996	113213	113314	115423	123010	142522	85714	85844	94321	90925	91511	91538	92542	94705	120920	121043	124210	110146	110355	105005	124812	142919	143037	144437
×	36675	36982	37799	37779	38986	36670	38214	36244	36077	38826	38276	36662	36467	37376	37448	37443	36659	38680	36044	37355	37199	37338	38423	39740	36525	37365	36626	38346	37830	39073	38576	38035	37846	38979
>	109119	108246	108835	109057	106600	106717	107749	108239	108207	106850	106485	106863	106870	107907	107495	109133	109579	109374	107286	107132	107872	107803	107786	107786	107723	107863	107104	108159	108728	106307	106536	108508	109034	106822
Time-out	113430	151855	104141	94213	95624	111518	133116	105143	105351	121848	133315	131307	124053	140833	141257	144012	134323	142211	133047	140836	112738	110201	140657	141319	143246	142622	135437	134121	115505	140945	140547	152432	153101	151430
×	37736	37371	37785	37707	38770	38794	38253	37464	37401	38844	38973	3608	36860	38066	37910	37582	37969	37998	38759	38853	37765	37773	3000	3 <u>8</u> 134	38087	84 84 84	36938	38076	37953	38757	38597	38077	37975	38934
>	108966	108145	108903	109030	106869	106890	107808	108912	108891	106861	106821	107010	106981	107911	107676	109104	108996	108997	107075	106976	108788	108771	107109	107099	107938	108039	107046	108787	108818	106954	106686	108752	108932	106912
Time-in	84538	125006	82523	85047	83826	83804	131550	90841	91039	114309	95537	113159	113251	115452	122940	142603	85724	85833	94305	99606	91438	91446	92604	94724	120947	121104	124205	110201	110319	105143	124806	142946	143106	144445
SISS	SS	ဟ	Ø	SS	Ø	SS	တ	SS	S	SS	ဟ	SS	ഗ	Ø	SS	SS	Ø	SS	တ	SS	တ	SS	S	SS	ဟ	SS	Ø	Ø	SS	Ø	SS	Ø	SS	Ø
Date	830227	830227	830304	830304	830304	830304	830304	830419	830419	830419	830419	830419	830419	830419	830419	830419	830423	830423	830423	830423	830424	830424	830424	830424	830424	830424	830424	830524	830524	830524	830524	830524	830524	830524

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Alpha-value	0.011791
K-value	0.011781 0.018038429 0.01908829 0.36281708 0.00710026 0.4838732 0.1691823 0.1691823 0.76257077 0.22810744 0.02576582 0.05464217 0.05748828
×	38971 36414 37055 38207 38044 37866 38610 38198 39410 39365 39365 39365 39365 39365 39365 39365 39365
>	107064 109774 107394 107394 109743 108842 108635 108983 107871 106946 106946 10697 106026 106101 106057
Thne-out	151102 120904 120028 140318 114312 145512 120342 145657 1144139 123249 130735 131245 123905 123905
×	38940 37735 38826 38854 37905 37906 37901 37962 38835 38101 38740 38740 38740
>-	107021 109348 109283 107122 108270 108126 108174 107193 106981 106981 106942 108334
Tíme-in	144326 93328 93220 94414 94529 92838 121353 94010 134406 94107 95317
×	38919 36381 37053 37053 37698 38028 38190 38407 39407 39407 39415 38826 38826 38826
>	107051 103838 103571 103740 103740 103622 103622 103622 103612 103632 103612 103612 103612 103612
Time-out	150838 120951 120009 140212 114301 145744 114732 145744 114732 145744 114732 123333 130806 131312 124065
×	38896 37693 37743 38782 37815 38301 38178 38178 38178 38174 38183 38763 38763 38763 38763
>	106393 106335 107185 107147 108257 108919 109212 109166 107314 106978 106923 10693 106849
Tme-in	144236 83334 94359 94539 92846 133041 125406 94001 134354 94113 95340 95331
SISS	88 88 88 88 88 88 88 88 88 88 88 88 88
Date	830524 830629 830629 830629 830701 830701 830701 830701 830701 830702 830728 830728

0.01457347 0.0005857161

TABLE A.2 Drogue measurements, K-values and Alpha-values for Hout Bay

Alpha-value	0 m4159735	O DOM DREATER	0.003062397	0.02503045	0.0203040	0.000000000	0.000196014	0.000196014	0.000236748	0.000130334	0.00084747	0.00064747	0.00028778	0.000220770	0.000103881	7.7288E.05	0.000318919	0.005482552	0.000735173	0.003985253	0.001176682	1 7127E-05	0.00090933	0.002438271	0.000507543	0.000469483	0.000944755	0.000117465	0.002042057	0.004296707	0.000195807	0.000328921	0.001328348	0.000100846
K-value	0.01799813	0.00558817	0.00486125	D 0.4721528	0.0287756	0.0523000	97707000	0.02/2000	0.02331701	0.02.43003	0.4889207	0.100231	0.09258299	0.14635007	0.0415005	0.00884367	0.05435424	1.31933411	0.20112622	0.45740582	0.24437891	0.0056945		-		_	_		-			-		_
×	71359	71186	70985	71228	70458	70754	2007	2007	5 £	74004	7.380	70093	69902	69707	70066	70409	70470	71304	71495	71300	70751	71492	71452	71609	70725	70488	70924	71945	70882	71550	70023	71242	71864	70830
> -	60289	60175	80199	60116	60037	60282	60226	6049	9208	60934	5044	58294	59406	59760	58219	60287	26566	89008	60470	90009	9009	59102	59651	59788	59011	60342	60177	96009	60697	59316	59626	90681	90380	60331
Time-out	125652	125942	130527	130247	154812	154511	154122	153839	141153	140842	150128	161607	162214	162742	161812	132948	134901	123745	152406	133057	154523	103156	95528	133123	111245	131424	121134	144909	104605	150206	163501	111130	103842	111641
×	70790	06/0/	70590	70590	70 <u>49</u> 4	70608	70860	70796	71129	70892	70067	70196	70211	20445	70211	70588	70515	70685	70899	71053	70557	71417	71321	70581	70832	70403	70838	72047	70787	71598	70289	71134	71303	70596
> -	90559	60550	60235	60235	60074	60275	90430	60524	90230	60841	59845	60252	59878	60457	59878	60458	59797	60215	80444	60254	60087	60114	80009	60630	59760	60286	59940	60014	60495	59343	558673	60646	60501	80151
Time-in	84845	84845	90006	90008	151129	135343	135723	135538	121909	130646	13314	104102	123756	112613	123756	111811	122250	93741	125000	125140	133735	92024	92146	115844	92450	122933	112826	135638	102341	141343	152909	104418	94748	92338
×	71374	71183	70968	71191	70475	70605	70953	70954	71158	70865	71484	70081	69768	69752	70032	70394	70636	71733	71533	71238	70617	71444	71372	71534	70637	70421	70904	71856	70810	71601	65656	711197	71779	70880
>	602.45	60149	60182	60175	90009	60295	80173	60466	60264	60636	59411	58297	50389	59619	58269	80308	59575	59974	90909	60082	60087	59160	59645	39904 4	28987	60352	8009	60059	99909	59456	59661	60585	60471	60377
Time-out	125744	130139	130652	125916	154901	154804	154218	153831	141238	140757	150013	161522	162354	162543	161714	132921	134804	124343	152619	133020	154429	103251	96704	132944	111424	131308	12/3/15	153235	104938	145828	163617	111210	103630	111408
×	70790	70790	70589	70589	70508	70586	70887	70829	71078	70858	71016	70242	70131	70426	70123	70618	70562	70742	70886	71032	70540	71359	71245	70616	70831	70359	70801	72025	70755	71621	70249	71098	71268	70650
>	80556	60556	60236	60236	90051	60269	90469	60541	90541	60826	59864	60290	59835	60434	59886	60476	59793	80183	60510	60282	60035	90 49 49 49	60013	60602	50706	60313	558873	59917	60463	26667	59952	90633	9000	601 23
Time-in	84838	84838	90001	900 D	151147	135324	135656	135603	121817	130636	133046	104210	123858	112633	123965	1174	122317	93700	124927	125124	133637	91932	92230	115807	92546	122843	112910	140050	102319	141250	152821	104353	94716	92302
SISS	တ	SS	SS	တ	တ	SS	Ø	SS	ഗ	SS	တ	S	SS	SS	Ø	SS	SS	SS	SS	ဟ	SS	SS	တ	ഗ	Ø	SS	SS	SS	SS	SS	တ	SS	တ	SS
Date	840607	840607	840607	840607	840607	840607	840607	840607	840608	840608	840704	840726	840726	840726	840726	840727	840727	840907	841003	841003	841003	841005	841005	841005	841006	841005	841029	841029	841029	841029	841031	841203	841203	841203

K-value Ainha-voine	
×	71707 70790 70656 70737 70680 71044 70801 70618 71170 69376 70719 71565 71716 71974 71922 69983 70696 71062
>-	60155 60215 60204 59865 59865 60454 60371 59633 60833 60833 60833 60833 60833 60833 60833 59635 60194 59685 59771 59800 59102
Time-out	103658 150914 150914 145910 145527 150555 145644 142317 144310 101931 111811 95317 133624 95317 134051 104244 12509 105846 105846 105846 105846 115822
×	71220 70552 70559 70720 70529 70744 70632 71072 71083 71137 71371 71371 71371 71371 71371 71371 71371 71371 71371 71371 71371 71371 71371 71371 71371 71371 71371 71371 71371
>	60212 60154 60347 58578 60249 60249 60415 60415 60415 60440 60440 59815 59855 59854 60347 60347 60347 60347 60347 60347 60347 59855 59853 59853
Time-in	95735 141052 140712 104356 120751 142045 114704 94836 1111027 92418 131149 92104 122451 102819 95140 120751 101621 101621 101621 101621 101621
×	71605 70732 70802 70808 70908 70908 71200 69845 71441 71463 71738 71738 71738 71748 71078 71078
>-	60232 60233 60253 56648 59728 60269 60269 60269 59433 60264 60264 59730 59730 59736 59736 59736 59736
Time-out	103542 150818 150508 151102 145250 14544 14244 144023 101809 114652 96865 133803 96865 133803 104921 104425 112738 112738 112742 104425 112738 112738
· ×	71126 70647 70615 70619 70717 70808 71089 71157 71467 71289 71289 71288 71289 71288 71288 71288 71288 71288 71288 71288 71288
>-	60281 60118 60360 60307 59672 60646 60258 60461 60818 60418 60528 59852 59852 59852 60406 59723 59725 59681 59723 59681
Time-in	95650 141029 140294 120651 142215 142245 114832 114832 114832 114832 114832 114832 114832 114832 115007 92031 131108 92267 131108 92267 127120 120853 95204 121120 1201805 101805
SISS	8
Date	841203 841205 841205 841205 841206 841206 850121 850328 850328 850328 850422 850422 850422 850422 850620 850620 850620 850620 850620 850620 850620 850620

TABLE A.3 Drogue measurements, K-values and Alpha-values for False Bay

Alpha-value	0.1107116093	0.0020649074	0.0002206495	0.005745825	0.1398203831	0,0016026662	0.0002246114	8.57225E-05	0.0014471425	0.0014109358	0.0018458797	0.001193554	0.0037082461	0.0044507915	0.0017211721	0.000515392	9.182063E-06	6.751762E-07	0.0033174571	0.0014506622	0.0001120863	1.183057E-06	0.0001454959	0.0003169967	0.0001822192	5.134306E-05	0.0005821221	0.0003126246	0.0002413417	0.0002389559	8.869482E-05	2.410527E-05	0.000673361	0.0046651447	0.0028893708
K-value	394.275551	0.33268211	0.05868077	2.14283045	56,2446321	0.2196069	0.04557188	0.04455108	16.0797181	0.47042498	0.64606446	0.20250555	1.27080038	0.62923914	0.13582185	0.23153846	0.00200884	0.00030126	-		_			m	0.02087314 (0.02720957	0.11709777 (0.08212845 (0.09773166	_	0.01148225 (0.01150837	0.1839172	1.1412	2.0127729 C
×	73893	76446	77637	76640	75723	75625	77027	74367	75135	75417	74075	74870	75990	79289	79795	77050	78304	73229	74920	75420	78933	76396	75792	73233	74359	74988	75815	74419	74344	73714	76339	73675	75487	76245	76940
 > -	34388	36430	22470	22040	22831	22330	22338	35019	35043	28278	28651	27442	26574	42264	43753	42490	44307	36206	36033	35116	22780	28841	28566	29463	30331	26348	25005	37571	36824	36875	27937	28057	27596	43900	43616
Time-out	161117	172546	121222	110646	112551	125341	132107	122638	123529	165411	155143	154429	152929	171810	183639	174248	182747	170641	172736	173449	124424	164211	141413	134617	170248	161743	160523	132832	131917	124554	115539	122058	124642	162754	163237
×	74302	76621	77471	76132	76983	75822	77380	74566	76080	76114	74588	78493	76341	78733	78639	77004	77861	76516	75002	77855	77877	77074	76620	74892	74924	76782	76528	74989	74427	74622	76491	74907	75313	76573	77714
>	37052	36175	23306	22648	23362	22635	23357	35788	35626	28596	27703	22314	27470	43858	44398	44265	45010	22870	36238	23509	23486	28423	28468	28079	28969	22081	22637	37314	36801	36679	28321	28746	27846	44003	43572
Time-ín	150024	162643	100103	94954	102849	113716	103133	104133	100610	152509	142644	120107	143446	132804	14043	134706	144635	104857	165033	94328	101453	121628	122124	105510	91101	113649	95607	123655	125203	110921	110736	102539	121118	150647	151920
×	73765	76505	77659	76900	76847	75526	76956	74362	74998	75575	73963	74722	76046	79553	79753	77067	78327	73196	74969	75238	78941	76332	75795	73155	74282	75003	75927	74355	74252	73645	76384	67757	75369	76440	22277
>	37629	36340	22461	21899	23324	22328	22859	34921	34946	28306	26506	27395	26762	42230	43634	42679	44253	36115	35943	35171	22769	28793	28466	29636	30335	26223	25110	37617	36854	36879	27957	28073	27594	43793	43514
Пте-out	170650	172658	121412	110841	112451	125446	131902	122405	123333	165304	155302	154547	152810	171647	183513	174444	182642	170737	172618	173344	124331	164005	141218	134801	1701	161500	160419	132721	131706	124421	115606	122002	124823	162927	163427
×	74027	76547	77515	76193	77071	75782	77327	74604	74998	76192	74526	78450	76391	78693	79513	27077	77851	76428	75027	77867	77898	76998	76551	74782	74889	76892	76486	74926	74337	74587	76535	74810	75246	76624	77833
>	37422	36138	23257	22589	23380	22637	23366	35686	35603	28594	27755	22333	27532	43867	4 383	4334	4884	22929	36203	23540	23463	28449	28419	29156	28968	22071	22670	37331	36808	36717	28336	28778	27851	43968	43507
Time-in	161222	162715	10001	82038	102921	113658	103110	104019	100527	152600	142854	120027	143529	132746	144142	134624	144957	104716	165015	94252	101437	121562	122235	105607	910 <u>4</u> 2	113532	98545 345	123747	125112	110850	110812	102422	121145	150607	152024
SS/S	တ	SS	SS	ဟ	S	SS	SS	SS	SS	Ø	တ	SS	တ	S	Ø	Ø	SS	SS	SS	S	SS	SS	Ø	တ	SS	SS	တ	Ø	SS	w	SS	Ø	SS	SS	တ
Date	870126	870126	870127	870127	870128	870128	870128	870129	870129	870129	870129	870129	870129	870202	870202	870202	870202	870203	870203	870203	870203	870204	870204	870204	870204	870205	870205	870206	870206	870206	870316	870316	870316	870316	870316

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TABLE A.4 Drogue measurements, K-values and Alpha-values for Viees Bay

K-value Alpha-value		0.28179477 0.002284472		_				Ξ.	0		~			10	_	0.36752577 0.001912008	_		_	_	_	0.29521552 0.00035879	_		_	_		_	_	_	_	۸.		0.07260271 0.000303167
	_	_	Ű,		_	_	_	_	_	_																								
×	9255	8826	8170	7859	8678	8211	<u>8</u>	10215	8252	8294	8362	8262	8677	8180	86 <u>4</u> 98	7651	7218	9019	8083	7463	7527	7298	7214	73 90 90 90 90 90 90 90 90 90 90 90 90 90	7484	73803	938	7271	8177	909/	8620	7989	368	9117
>	90006	88415	89166	88219	87281	86143	89755	91410	89130	90626	90850	90284	88873	86588	80882	88005	87630	89819	06806	86397	85650	84915	84147	85829	84807	84660	84433	86793	86970	86375	86613	86203	88291	85850
Time-out	144548	121904	141939	122521	140130	145022	143913	134649	141959	122426	134804	114815	152030	140355	145428	123133	120429	125054	132957	124918	125356	144705	131317	113656	130523	114738	141503	134408	144138	131336	145034	142824	132207	150155
×	8884	8857	8032	7978	9102	8208	93395	95 40 40 40 40 40 40 40 40 40 40 40 40 40	8108	8106	9940	8379	9663	8472	8122	7734	7504	10006	8407	7512	7560	8003	7223	7386	7718	7700	8483	7302	8123	8011	8750	8636	7678	9147
>	87495	87712	86980	87094	88365	89138	89113	88460	88066	88874	90194	90459	89781	89724	88494	88540	88338	90128	90837	86392	85538	85838	85662	80008	82608	85803	85188	86950	86726	86467	86313	86210	87152	85960
Time-in	113442	113716	112651	112844	125500	123834	133409	112342	121946	105421	113457	103059	113414	132212	104835	115802	112540	94625	124149	121857	122234	125835	122613	111410	111652	104632	130816	114743	124812	110824	121919	111827	115359	113636
×	9276	8742	8295	778 <u>6</u>	8597	8118	9515	10127	8169	8353	9188	8088	8628	8157	6510	7626	7257	8720	8091	7584	7518	7187	7161	7322	7518	7468	7893	7347	8162	7470	8588	8012	7992	9221
>	83858	88417	98114	88234	87365	86194	89760	91171	89109	90603	91015	90161	89065	89479	80783	87917	87810	89826	90840	86300	85763	84750	85128	85757	84846	84863	84486	86849	87002	86489	86630	86082	88189	85849
Time-out	144358	121938	142046	122563	140035	144831	143749	134511	141819	122554	135018	114952	151827	140431	145512	122941	120322	124838	132913	124759	125317	144943	131442	113827	130331	114614	141354	134309	144240	131221	144910	142956	132315	150002
×	8893	8828	8049	7956	9026	8463	908	9430	8062	84	9854	8290	86 44	8463	8158	7707	7510	2867	8443	7617	7547	7942	7191	7350	95/	7749	8451	7362	8094	7976	8733	8966	7738	9192
>	87516	87735	87038	87122	88429	89176	89138	88442	88074	88817	90252	90385	89877	89652	88489	88496	88453	90115	90699	86348	86028	85697	85696	85987	85909	85915	85227	86992	86788	86480	86392	86134	87077	86001
Time-in	113459	113734	112755	112905	125531	123722	133455	112405	122000	105358	113539	103209	113320	132245	104900	115721	112721	94602	124208	121942	122159	125712	122631	111351	111719	104548	130750	114830	124859	110753	121985	111750	115611	113559
SISS	SS	U,	SS	(0)	Ŋ	SS	SS	ဟ	SS	Ø	SS	SS	SS	Ø	S	SS	(C)	SS	SS	S S	S	v.	SS	SS	(C)	ဟ	Ø	V.	8	, co	G.	ď) <i>(</i> 0	SS
Date	851118	851118	851118	851118	851120	851120	851123	851123	851123	851127	860129	860129	860201	860201	860204	860204	860204	860207	860207	860410	860410	860410	860414	860415	860415	860415	860416	860418	860418	860418	860418	860418	860424	860424

Alpha-value		0.000305226	6.7651E-05	0.006495888	0.000998938	0.005244882	0.000293451	0.001609158	0.010772134	0.003048597	0.022801759	0.000480544	0.000100041	204700000	0.0002032	2000180012	3.8413E-05	0.000750462	9.9892E-05	0.000431907	0.00050635	0.000330706	0.000165788	5.874F.05	Cascarcion o	000 4F0B6	U.UUU45888	0.000421623	0.000751938 0.000751938
K-value			0.01378594	0.6955554 (0.20912447 (1.62231414 (0.03792135 (0.35653245	_										0.02355072	0.24387138 0	0.12444688 0	0.15591582 0.	0.0434308 0.	0.0203655	_				0.29353787 0.1
×		8230	8048	8572	9418	9753	7917	8256	8827	7543	8244	8060	7872	9233	8264	7034	3 6	द उ१४	7487	7324	7152	7429	6910	8919	9448	7074	787	1801	7365
>		25.55	88676	85507	86014	85535	85381	86661	84884	86846	85398	87632	86629	87946	85090	8270E	3 2	\cco.	87157	88193	82372	84381	86074	85784	87083	88375	R7304	88044	88292
Time-out	40,400	4240	110848	121831	123740	114851	144925	143818	130458	131550	114820	142320	123132	115224	143848	153239	196720	25/55	34042	133555	170709	112633	125940	142159	125665	124444	123725	115841	
×	27/20	? [? ? ?	ĝ	7926	<u>8</u>	9686	8274	8783	933	7746	8135	8232	8108	9274	8920	9006	828	3 8	570/	0/4/0	8 8 8	8071	7982	906	9197	7918	7979	7967	45
>	87042	270070	00/0	963/0	86208	85896	82581	86476	86208	86514	86226	86534	86574	87306	85918	85948	87001	5000	00000	8/036 0 11 11	84/43	85189	86310	85829	86334	87625	87035	86974	87913
Time-in	133452	0584B	5 (S	2570	1112441	111047	140833	120138	91413	113548	96438 85438	113412	113706	111884	112836	101411	113959	132A		124025	13,0229	/2007/	100209	115952	105927	114657	114350	91827	114539
×	8192	07.08	2 60	÷ 6	9524	7706	9	8104	0808	7576	8543	윤 88	7966	9228	8290	7032	8431	75.36	} {	177	9 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	£ 8 ₹ 8	900	8877	9487	8118	7888	7949	
>	87382	88730	953		2000	0.00	20,000	880/C	/B508	86626	8	8/681	86878	88005	85152 22	82735	88382	87177	6,7	2/20	05770	70740	7/000	85864	86941	88430	87303	88026	88154
Time-out	142306	110926	122027	123010	11.4056	0 10 0 1 0 0 0 0	4004	1940	13024	131700	114000	142453	123255	115245	143700	153220	136548	134848	133336	170030	112748	100000	7,0002	142040	125846	124649	123646	115801	130545
×	8183	8010	7044	9	2 6 2 8	} 6	2 20 0	0 20	0 1	1077		# 1	81/8	1976	888	<u>8</u>	8632	549	7419	8337	8077	. KOV	0 0	0.67	8500	8 74	7862	7952	7532
>	4	_) occusion																			
Time-In	133355	95514	102231	111225	111109	14000	120114	9453	115,650	05.02	113310	11000	144057	708111	08211	151430	114025	123621	124115	130455	102701	10028	130406	2002	4000	114736	114212	91722	114513
SISS	SS	Ø	တ	SS	Ø	SS	SS	ď	v.	} v:	U.	} u	ο υ) {	200	n	S	SS	S	SS	တ	Ø	Ü	3 4	0 (o,	SS	SS	SS
Date	860424	860425	860718	860718	860718	860723	860725	860725	860725	860725	860726	RAN728	Sen728	960700	02/120	97/000	860729	860728	860728	860731	861004	861007	884044	28001		1010	361011	361015	361016

TABLE A.5 Drogue measurements, K-values and Alpha-values for East London

K-value Alpha-value	0.01678043 0.000265283				_					~		_	0.4946033 0.003808458		0.100/323/ 0.0021/90/8							0.00570896 3.89134E-05	0.08470837 0.00049524		0.52198657 0.012946907	0.01311224 3.08443E-05	_			_			_	_	0,25810238 0,00371969
×	59198	594%	59m3	£00.48	29240	34000 18000	99058 13409	30 Ago	30213	58,00	20409	2000	58440	2 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	52387								58871	59051	59270	55379		52386		54271	53757	54010	59150		58434 0
>	-83753	83507	84863	2 T/2 C	2007	97.00	2070	97637	(CD/O	940	2004.0 2004.0 2004.0	84745	83867	92686	6506	9 48 4	78778	80737	0.000	9 6	040 040 040	83890 0890	84798	-84190	9455 9	-88413	-88334	-90482	-89880	-88516	-88598	-88528	-88222	-85737	-85512
Thne-out	91645	90636	103739	40270B	25.75 CENTER	1814/0	12532	666	10472	130700	85.00 C	85620	114556	101423	103609	90504	101327	120247	173878	12020	900	100621	114140	113130	112700	112221	110454	114315	115146	130318	125907	131644	100532	94026	94206
×	58475	58475	57968	17068	5.4877	5.48.77	7.40 8.40 8.40 8.40 8.40 8.40 8.40 8.40 8	7,405.8	57.73	58780	58188	28188	58628	53604	53545	54319	53785	53872	121.47	3 2		28660	58105	58105	58523	₹ 4	55144	53950	53950	54516	54593	54593	59105	58136	58136
>	-85422	85422	-86879	-86879	-90033	9003	803	-RGTR4	85047	-84504	-85217	-85217	-84547	-90250	89656	90934	-89046	89116	288735	95913	3 6	85813	82 038	-82038	-85497	8868	86638	89068	89068	-89489	-88510	-88570	-86122	-85038	-85038
ТІте-in	74724	74724	84849	84849	100625	100625	100751	102751	93338	123347	74441	74441	103712	82023	83924	81644	80447	80412	102622	8/000	62010	84029	85145	85145	102916	84147	84147	92847	92847	120218	120839	120839	81159	83035	83035
×	59163	59402	59083	59196	54539	35097	53520	55077	57616	58383	57861	58064	58411	51736	52359	53898	53504	53366	54702	67952	3 6	0000	58874	58915	29267	88	26400	52362	53147	54262	53752	53858	59203	58612	58382
>	-83749	-83563	-84891	-84614	-87659	87354	-88783	-87736	-85515	-84113	-84567	-84582	93950	-92759	-90739	-91675	-88897	-89214	87948	84005	2000	3	-84703	-84327	-84673	-88345	-88328	-90506	-89968	-88470	-88628	-88584	-88094	-85708	-85429
Time-out	91502	90742	104021	102643	155506	161002	125256	160150	101619	130738	85434	85803	114515	101600	103301	82906	101402	120148	123855	100425	100700	80/001	113815	113410	111944	112421	110923	114158	114957	130355	125835	131852	100339	94040	94236
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Alpha-value		6.27302E-05	0.000817655	0.000830742	0.001900304	0.000252587	0.000188754	0.001609188	0.001715825	0.001283545	0.001767943	0.129215888	0.000342418	0.00148900	0.004044747	0.004941/4/	0.W2U18663	0.000874741	0.000336549	5.47283E-05	0.003893487	0.002194102	0.000235571	0.001300148	0.000108047	0.000000014	0.001023714	0.002263011	0.00085753	0.003482475
K-value		0.00/66558	0.17551622	0.1412037	0.26961713	0.0144567	0.04051745	0.21515246	0.16053585	0.1263183	8.06967905	1286165.81	0.09190242	0.13040897	0.48022058	0.14140000	0.14115569	0.16814253	0.02716068	0.00232558	0.94561856	0.56798051	0.04459911	0.18468468	0.07458597	0.3707004	0.306/8001	0.35000059	0.04001136	0.74865
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×	58137	CB267	70707	500	58778	59672	22 122	2206 6	59248	57214	54697	53708	23830	57544	28069	58608	58507	57777	1	27//6	54976	54975	54085	54178	58638	54183	53877	53344	53181	54639
>	-84665	84500	2 6	04/40	84196	84 88	89764	-89576	-85724	-86243	88455	88585	-88366	85105	-84522	-86426	-86637	86417	1 600	B) 00	-90162	90545	89025	89194 94	-86009	-90840	-90375	90141	-88874	-88227
Time-in	75341	75114	10000	20002	4 6	112959	75526	81641	104438 88	115022	104822	115240	111605	134804	135210	90010	90213	54070	0,4876	992	8 5	91/33 33	9000	90352	123116	90302	105704	102315	121026	131903
×	58774	59156	£833	2 2 2	8 8		3 3 8 8	22 152	2963	56587	35148	53835	54773	57642	58982	58202	57655	57424	27.40	2 10 0	7 i	24/23	24245	24 23	58103	53507	52246	52970	53155	54979
>	-83797	-83592	84474	93700	9 66	04023	92700	68288 28288 48288	85184	8/534	3	88545	8/786	84815	-83706	-86862	-87720	-86824	87778	79300	, oons-	2 12	\$ 68 68 68 68 68 68 68 68 68 68 68 68 68 6	79095	-85866	91750	-91357	-90579	-88925	-87838
Time-out	84338	83614	114717	115407	13,55	120103	126221	100401	120554	141905	0.00	124356	90507	141850	145238	93022	100037	101641	101145	10284B	100040	25.55	# 1507 # 1675 # 1675	3	42208	94154	115207	105218	122035	140108
×	58151	58224	58117	58739	50680				8228 7274	0,440 10,40 10,40	0474	2/0/2		00/0	9 9 9	58611	58551	57724	57713	55037	(A)	5,4127	12 15 15 15 15 15 15 15 15 15 15 15 15 15		98880	8 <u>118</u>	5339	53330	53189	54583
>	-84631	-84626	-84736	-84209	84451	-80708	BOE 04		BE244	02720		-00024 88.724	12400	2000	94500	-86402	96610	86403	-86665	02170	-00534	98008	8915.4		900	6/ 9 06	90329	90170	88867	88223
Time-In	75323	75102	104941	111009	113010	75.44	81653		115032			111533																_	_	131928
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APPENDIX B

DETAILED STATISTICAL ANALYSES AND PLOTS OF DATA

ALL STUDY SITES (GROUPED)

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			Z.		0.0000068		0.0000078	0.00000068	0.0000012	0.0000078	0000000	annonnon.	0.0000078	0.0000012	1	0.000015	0.00001	0.0000078	0.0000012	0 000047	0.000013	0.0000092	0.0000012	0 000015	2 Constant	0.0000078	0.0000012
			Max		0.0266		0.0266	0.0109	0.0266	0.008	0.013		0.0069	0.0266	0.053	200	0.0266	0.0056	0.00424	0.0286		0.0108	0.01295	0.013	20,50	0.0109	0.0256
	2-VAL 165	מ-אשרח	Std Dev		0.0022	0000	0.0026	0.0015	0.00258	0.00136	0.0023		0.0012	0.0029	0 0024		0.0033	0.00132	0.000793	0.00332	0,000	0.0018	0.0020	0.0016	0.0018	0.003	2000
			Median		0.00058	0.00062	70000	0.00051	0.00046	0.00054	0.00083	0.00055	0.00000	0.00050	0.00062	0.000	6/0000	0.00054	0.000361	0.000727	0 0000	0.0000	0.00074	0.00082	0.00051	0.00045	<u>.</u>
1		Moon			0.00131	0.0015	777000	0.00114	0.00127	0.00106	0.0017	0.00103		0.0014	0.0016	0,0040	8100.0	0.00113	0.00066	0.00153 (0.00135	_	0.0015	0.0013 (0.00129	0.00138 0	NU.
		Z. Z.			0.0003	0.00084	0.0003	2000	0.00041	0.0016	0.0003	0.0013	1300	0.00041	0.0016	0.00084		0.0018	0.00041	0.00155	0.0019		0.00041	0.0014	0.0013	0.00041 0	4
	1	Max			1.92	1.27	1.92		78.	2	1.27	1.2	5	7	1.1	1.038		1.271	1.92	1.195	1.195	0 27	\dashv	.1.27	1.919	1.066 0	
	N- VALUES	Std Dev			0.2809	0.2642	0.2972	0 3044	0.00	0.2366	0.2785	0.2800	03060	2000	0.2614	0.1781	╀	-	0.3486	0.2603	0.3057 1	0.2853	+	0.2837	0.3104	0.2342 1.	
		Median			0.1050	0.1197	0.0931	0.0841	0 4400	001 100	0.1416	0.1042	0.0856		U.1263	0.0547	0 1220	0.1220	0.1782	0.0865	0.1252	0.2104	- -	0.1429	0.0931 (0.0793	
	1	Mean		3000	0.2205	0.2175	0.2235	0.2038	0.2256	0.2370	0.43/3	0.2228	0.2157	0 2260	0.5.200	0.1298	0.2342	71,777	0.2891	0.1192	0.2511	0.285	+	+	0.233	0.1734 (
		S	<u>(</u>		7 - F		ALL	ALL	ALL	AH		> 200	500-1000	> 1000	200	ALL	ALL		71.	1	ALL	ALL	=	1	ALL	ALL	
		<u>ا</u> ل	Ē	- A	N I V		ALL	ALL	ALL.	AE			ALL	ALL A		4. 6	40-80	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	3 1		ÀLL ALL	ALL			+	ALL	
CONDITIONS	9	s/ss		S/SS	S		SS	S/SS	S/SS	sys	00/0	3	S/SS	S/S		2000	S/SS	8/8	88/8		00/0	S/SS	S/SS	+-		S/SS	
CON	F	(hrs)		ALL	ALL		ALL	ALL	ALL	ALL.	Ī	+	ALL	ALL		7	ALL	ALL	ALL	╁	- -	ALL	^ 1 v	1.0	+	22	
	Cimon	ts	(m s ⁻¹)	ALL	ALL		ALL	< 0,1	0,1-0,2	> 0,2	A		AFT.	ALL			ALL	ALL	ALL	1.14	_	ALL	ALL	114	-	ALL	
	Wind.	(knts)		ALL	ALL	110	1	ALL	ALL	ALL	ALL		7,1	ALL	1 A	1	AL.L	ALL	, 5	5-15		× 15	ALL	At		7	
C	Cione, y ₃	i distribution de la constante		404	205	90		158	137	109	159	1	<u>+</u>	131	136		192	76	2	193	+	2	133	146	+	83	

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SAMPLE		CC	CONDITIONS	S				K- VALUES					α-VALUE		
*	Wind	Currents (m s ⁻¹)	SS/S	رس)	(w) S	Mean	Median	Std Dev	Max	Min	Mean	Median	Std Dev	Max	Min
147	ALL	ALL	S/SS	ALL	ALL	0.152	0.0667	0.2377	1.92	0.00084	0.00096	0.000375	0.00162	0.0116	0.0000078
79	ALL	ALL	s	ALL	ALL	0.1295	0.073	0.1632	0.923	0.00084	0.00098	0.00039	0.00162	0.0116	0.0000078
67	. ALL	AĽL	SS	ALL	ALL	0.1781	950'0	0.301	1.92	0.0013	0.000952	0.00036	0.00136	0.0109	0.000016
77	ALL	< 0,1	ss/s	ALL	ALL	0.152	0.0593	0.266	1.92	0.00084	0.000919	0.00036	0.00164	0.0116	0.00001
40	ALL	0,1-0,2	S/SS	ALL	ALL	0.1367	0.0564	0.197	1.004	0.0016	0.00067	0.000365	0.000845	0.00366	0.0000076
26	ALL	> 0,2	SS/S	ALL	ALL	0.176	0.1103	0.205	0.923	0.0034	0.00157	0.00058	0.0023	0.0109	0.000034

FALSE BAY

								THE PERSON NAMED IN COLUMN TWO IS NOT THE PERSON NAMED IN COLUMN TWO IS NAMED IN COL							
SAMPLE		ខ	CONDITIONS	S.			:	K- VALUES					α-VALUE		
#	Wind	Currents (m s ⁻¹)	s/ss	(m)	S (m)	Mean	Median	Std Dev	Max	Min	Mean	Median	Std Dev	Max	Min
51	ALL	ALL	S/SS	ALL	ALL	0.2593	0.1252	0.3203	1.27	0.00031	0.00096	0.00037	0.00123	0.0047	0.00000068
20	ALL	ALL	S	ALL	ALL	0.2239	0.1248	0.2962	1.27	0.0116	0.000916	0.000317	0.00116	0.00445	0.000024
31	ALL	ALL	SS	ALL	ALL	0.2834	0.1256	0.3335	1.14	0.0003	0.000986	0.00038	0.00127	0.00409	0.0000068
14	ALL	< 0,1	SS/S	ALL	ALL	0.3521	0.1358	0.3947	1.14	0.00041	0.00148	0.000901	0.00151	0.00469	0.000012
24	ALL	0,1-0,2	S/SS	ALL	ALL	0.2169	0.1171	0.2291	0.867	0.011	0.00071	0.00033	0.00101	0.00445	0.000024
13	ALL	> 0,2	SS/S	ALL	ALL	0.2317	0.10344	0.354	1.271	0.0003	0.00081	0.00030	0.00105	0.0037	0.00000068

HOUT BAY

	i	ರ	CONDITIONS	٨s			A 編集	3							
>	Wind	1	00.0					N- VALUES		i			α-VALUE		
• :	}	(m s ⁻¹)	2/22	⊒ (E)	s (E	Mean	Median	Std Dev	Мах	Min	Mean	Median	Std Dev	XaX	1
Ī	A! I	Ali	000										- Neciona		
: 1	1	AEL	2/22	ALL	ALL	0.2269	0.112	0.2514	0.076	00,000					
₹	ALL	All	٥					110000	0.970	0.00486	0.00173	0.00077	0.0036	0.0266	0.000017
1		,	,	ALL	ALT.	0.285	0.227	0.265	0.976	0.018	2000				1100000
₹	ALL	ALI	v	-						2010	0.00243	0.00091	0.00509	0.0266	0.000094
ĺ	+		3	YET.	ALL	0.1893	0.0809	0.2349	0 9307	0,000	00000				
ALL		< 0.1	88/8	-	- : «				2000	0.0049	0.00123	0.00074	0.0015	0.0069	0.000017
1	\dagger			ļ	Y.L.	0.1587	0.0594	0.2215	0.9307	0.0049	50000	0.00072	! 55 6		
₹	ALL	0,1-0,2	SS/S	ALL	14	03080	0000		1			0.00073	0.0047	0.0266	0.000077
3	-					0.0000	0.3002	0.2697	0.9756	0.0213	0.0014	0.00088	0.0013	1,000	
7	-	> 0,2	S/SS	ALL	ALL	0.3185	0 3882	0 33 40					Closes	0.0044	0.000091
							2000	0.2349	0.7557	0.0056	0.00127	0.00124	0.00098	0.0024	0.000047
															2.0000.0

VLEES BAY

			ľ			j									
SAMPLE		ັ ວ	CONDITIONS	NS				K. VALUES							
*	Mind			L				N- VALUES					a-VALUE	,,,	
		(m s ^{-t})	8/82	<u>ع</u> د	s (E	Mean	Median	Std Dev	Max	Min	Mean	Median	Std Dev	Max	, M
54	Ą	Δ13	00/0												
		7	00/0	ALL	ALL	0.2778	0.1632	0.3067	1 195	93000	00000				
74	-	<	,						3	0.000	0.00108	0.00051	0.0014	0.0065	0.000027
	1	7	0	ALL 	ALL	0.2981	0.2064	0.3291	1 105	0.00					
5	-		6						<u> </u>	0.000	0.00121	0.00046	0.0016	0.0065	0 000027
3	1	ALL	2	ALL	ALL	0.2608	0.1438	0 2855	90,						700000
ά	74	, 0,	000					2007.2	90	0.0204	0.00098	0.00052	0.0012	0.0053	0 000057
2	7,	- - - -	500	ALL	- Arr	0.2745	0.1566	0.2038	7 700	. 0000					20000
20	110	0,00	200					0:52:0	3	0.0204	0.00088	0.00036	0.00105	0.00404	0.000057
	1	2,1-0,2	200	- ALL	ALL	0.2561	0.1848	0.252	0 001	0.000	0.000111				
ű	-		00.0						50.0	90000	0.00111	0.00048	0.0017	0.0065	0.000038
	1	7.0.7	2/22	ALL	ALL	0.3119	0.1559	0.3689	Ç	, ,			†	†	
									2	٠.U	0.00126	0.00061	0.00136	0.005	0.000027
						٠									

EAST LONDON

SAMPLE		00	CONDITIONS	SI				K- VALUES					α-VALUE		
#	Wind	Currents (m s ⁻¹)	ss/s	(ш) T	s (m)	Mean	Median	Std Dev	Мах	Min	Mean	Median	Std Dev	Мах	Min
93	ALL	ALL	SS/S	ALL	ALL	0.2695	0.1564	0.2981	1.195	0.00155	0.00193	0.0011	0.0024	0.01295	0.000015
52	ALL	ALL	S	ALL	ALL	0.2843	0.1681	0.2995	1.183	0.00155	0.00219	0.00112	0.00273	0.01295	0.000015
41	ALL	ALL	SS	ALL	ALL	0.2504	0.1304	0.2953	1.200	0.00185	0.00159	0.00109	0.00173	0.0078	0.000039
15	ALL	< 0,1	ss/s	ALL	ALL	0.3264	0.1498	0.3903	1.200	0.0221	0.00176	0.00116	0.002	8200.0	0.0001
31	ALL	0,1-0,2	ss/s	YTY	ALL	0.2908	0.1605	0.3031	1.04	0.0016	0.0017	0.00123	0.00167	800.0	0.000015
47	ALL	> 0,2	SS/S	ALL	ALL	0.2361	0.1566	0.2506	0.975	0.00185	0.00217	0.00088	0.0028	0.01295	0.000039

Plots of α -values for all the data versus current velocity, distance from discharge location, length scale, wind speed and time and water depth.

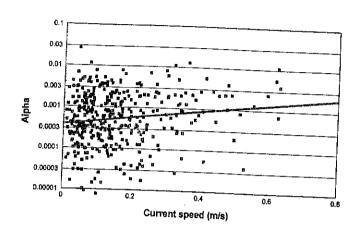


Figure B.1 α-values (log scale) for all sites versus current speed

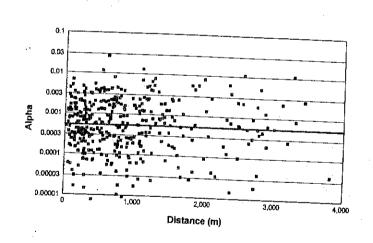


Figure B.2 α -values (log scale) for all sites versus total distance from discharge point

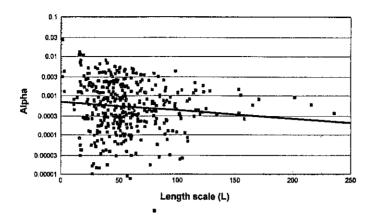


Figure B.3 α-values (log scale) for all sites versus length scale (L)

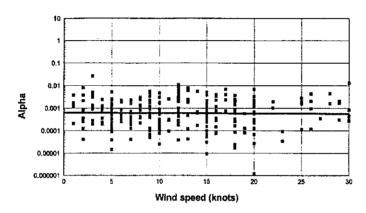


Figure B.4 α-values (log scale) for all sites versus wind speed

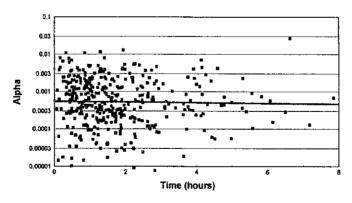


Figure B.5 α -values (log scale) for all sites versus time

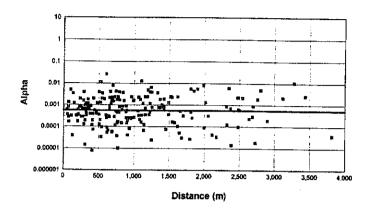


Figure B.6 α-values (log scale) for all sites (surface) versus distance

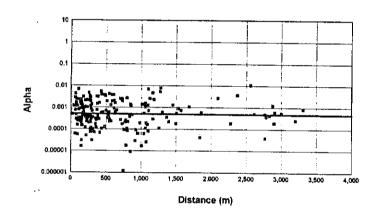


Figure B.7 α -values (log scale) for all sites (-5 m) versus distance